

HISTORIC AND DESIGN REVIEW COMMISSION

April 03, 2024

HDRC CASE NO: 2024-119
ADDRESS: 600 HEMISFAIR PLAZA WAY
LEGAL DESCRIPTION: NCB 13814 BLK 3 LOT PT OF 12 ARB 12G (GIS AC 3.847 AC)
ZONING: D, RIO-3, H
CITY COUNCIL DIST.: 1
DISTRICT: Hemisfair Historic District
APPLICANT: Andres Andujar/Hemisfair Park Area Redevelopment Corp
OWNER: Hemisfair Park Area Redevelopment Corp
TYPE OF WORK: Construction of a covered sports court
APPLICATION RECEIVED: March 15, 2024
60-DAY REVIEW: May 14, 2024
CASE MANAGER: Edward Hall

REQUEST:

The applicant is requesting a Certificate of Appropriateness for approval to construct a sports court and open-air pavilion in the planned Tower Park at Hemisfair, in the location of the existing pond. The proposed court will feature a concrete slab surface measuring one-hundred six (106) feet by sixty-two (62) feet, a roof structure and an overall height of approximately twenty-nine (29) feet.

APPLICABLE CITATIONS:

Unified Development Code Section. 35-642. New Construction of Buildings and Facilities.

In considering whether to recommend approval or disapproval of a certificate, the historic and design review commission shall be guided by the following design considerations. These are not intended to restrict imagination, innovation or variety, but rather to assist in focusing on design principles, which can result in creative solutions that will enhance the city and its neighborhoods. Good and original design solutions that meet the individual requirements of a specific site or neighborhood are encouraged and welcomed.

(a) Site and Setting.

- (1) Building sites should be planned to take into consideration existing natural climatic and topographical features. The intrusive leveling of the site should be avoided. Climatic factors such as sun, wind, and temperature should become an integral part of the design to encourage design of site-specific facilities which reinforces the individual identity of a neighborhood and promotes energy efficient facilities.
- (2) Special consideration should be given to maintain existing urban design characteristics, such as setbacks, building heights, streetscapes, pedestrian movement, and traffic flow. Building placement should enhance or create focal points and views. Continuity of scale and orientation shall be emphasized.
- (3) Accessibility from streets should be designed to accommodate safe pedestrian movement as well as vehicular traffic. Where possible, parking areas should be screened from view from the public right-of-way by attractive fences, beams, plantings or other means.
- (4) Historically significant aspects of the site shall be identified and if possible incorporated into the site design. Historic relationships between buildings, such as plazas or open spaces, boulevards or axial relationships should be maintained.

(b) Building Design.

- (1) Buildings for the public should maintain the highest quality standards of design integrity. They should elicit a pride of ownership for all citizens. Public buildings should reflect the unique and diverse character of San Antonio and should be responsive to the time and place in which they were constructed.
- (2) Buildings shall be in scale with their adjoining surroundings and shall conform to the identifying quality and characteristics of the neighborhood. They shall be compatible in design, style

and materials. Reproductions of styles and designs from a different time period are not encouraged, consistent with the secretary of the interior's standards. Major horizontal and vertical elements in adjoining sites should be respected.

(3) Materials shall be suitable to the type of building and design in which they are used. They shall be durable and easily maintained. Materials and designs at pedestrian level shall be at human scale, that is they shall be designed to be understood and appreciated by someone on foot. Materials should be selected that respect the historic character of the surrounding area in texture, size and color.

(4) Building components such as doors, windows, overhangs, awnings, roof shapes and decorative elements shall all be designed to contribute to the proportions and scale of their surrounding context. Established mass/void relationships shall be maintained. Patterns and rhythms in the streetscape shall be continued.

(5) Colors shall be harmonious with the surrounding environment, but should not be dull. Choice of color should reflect the local and regional character. Nearby historic colors shall be respected.

(6) Mechanical equipment or other utility hardware should be screened from public view with materials compatible with the building design. Where possible, rooftop mechanical equipment should be screened, even from above. Where feasible, overhead utilities should also be underground or attractively screened. Exterior lighting shall be an integral part of the design. Interior lighting shall be controlled so that the spillover lighting onto public walkways is not annoying to pedestrians.

(7) Signs which are out of keeping with the character of the environment in question should not be used. Excessive size and inappropriate placement on buildings results in visual clutter. Signs should be designed to relate harmoniously to exterior building materials and colors. Signs should express a simple clear message with wording kept to a minimum.

(8) Auxiliary design. The site should take into account the compatibility of landscaping, parking facilities, utility and service areas, walkways and appurtenances. These should be designed with the overall environment in mind and should be in visual keeping with related buildings, structures and places.

FINDINGS:

- a. The applicant is requesting a Certificate of Appropriateness for approval to construct a sports court and open-air pavilion in the planned Tower Park at Hemisfair, in the location of an existing, man-made pond. The proposed court will feature a concrete slab surface measuring one-hundred six (106) feet by sixty-two (62) feet, a roof structure and an overall height of approximately twenty-nine (29) feet.
- b. DESIGN REVIEW COMMITTEE – This proposal was reviewed by the Design Review Committee on March 12, 2024. At that meeting, Commissioners asked questions regarding the proposed design, materials, lighting and future landscaping.
- c. EXISTING SITE – The applicant has proposed to construct the proposed sports court in the location of the existing pond, which will require the demolition of the existing elements. The pond was constructed in 1988, is no longer functioning and is not contributing to Hemisfair. Staff finds its removal to be appropriate.
- d. EXISTING SITE – The existing site features a number of existing trees. The applicant has noted coordination with the City Arborist office regarding tree preservation. Additionally, the applicant has noted the planting of two new trees on site.
- e. COURT & PAVILION (Footprint & Massing) – The applicant has proposed to construct a sports court to feature a concrete slab surface measuring one-hundred six (106) feet by sixty-two (62) feet, a roof structure and an overall height of approximately twenty-nine (29) feet. Staff finds the proposed footprint and overall height of the court and pavilion to be appropriate.
- f. PAVILION (Materials) – The applicant has proposed for the pavilion to feature materials that include metal columns, soffits and roof decking. The applicant has noted ceiling material options to include metal panels, PVC panels or high-density polyethylene. Generally, staff finds the proposed materials to be appropriate. Final materials selections should be submitted to OHP staff for review and approval.
- g. PAVILION (Lighting) – The applicant has submitted a reflected ceiling plan that notes the locations of lighting fixtures. Additionally, the applicant has noted both uplighting and downlighting from the structural columns. Staff finds the proposed lighting to be appropriate and that lighting, as proposed, will not create light pollution throughout Hemisfair.
- h. LANDSCAPING – Detailed landscaping plans have not been submitted at this time. Staff finds that any future landscaping or seating elements be submitted to OHP staff for review and approval. These plans should be

developed to complement future hardscaping and landscaping plans for this section of Hemisfair and Tower Park.

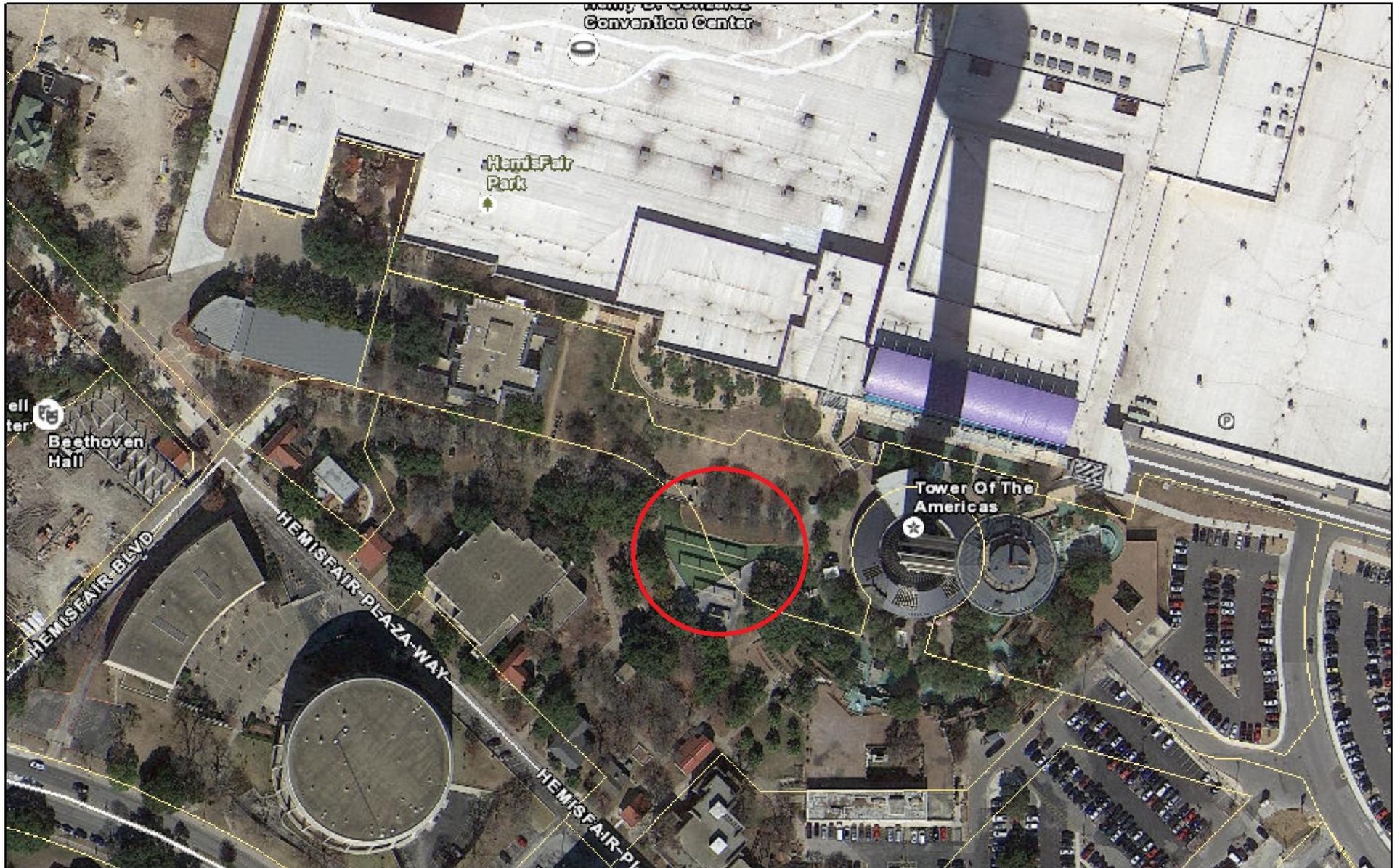
- i. ARCHAEOLOGY – An archaeological investigation is required. The project shall comply with all federal, state, and local laws, rules, and regulations regarding archaeology, as applicable. Work within public property is subject to the Texas Antiquities Code.

RECOMMENDATION:

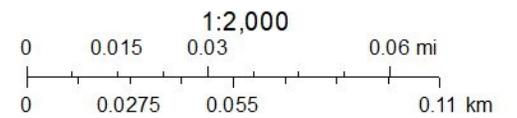
Staff recommends approval based on findings a through g with the following stipulations:

- i. That final ceiling materials specifications be submitted to OHP staff for review and approval.
- ii. That any future landscaping or seating elements be submitted to OHP staff for review and approval. These plans should be developed to complement future hardscaping and landscaping plans for this section of Hemisfair and Tower Park.
- iii. ARCHAEOLOGY – An archaeological investigation is required. The project shall comply with all federal, state, and local laws, rules, and regulations regarding archaeology, as applicable.

City of San Antonio One Stop



March 27, 2024





CITY OF SAN ANTONIO
**OFFICE OF HISTORIC
PRESERVATION**

Historic and Design Review Commission
Design Review Committee Report

DATE: March 12, 2024

HDRC Case #: 2024-119

Address: Hemisfair Park

Meeting Location: Webex

APPLICANT: Andres Andujar

DRC Members present: Jeff Fetzer, Monica Savino, Roland Mazuca, Jason Vasquez

Staff Present: Edward Hall, Cory Edwards

Others present: Gary Boyd, Melissa Chamrad

REQUEST:

Construction of a basketball pavilion at Hemisfair Park

COMMENTS/CONCERNS:

AA: Overview of request, concept and background of design, location of pavilion (at pond location; pond constructed in 1988). The pond is no longer functional; the equipment is in disrepair. Water is stagnant and not flowing.

MS: Questions regarding landscaping – will additional trees be planted? (AA: Yes, two new trees; many existing trees that will be protected. In discussion with City Arborist regarding impacts to two potential trees; will mitigate.) MS: Please provide shade studies and how that will impact the trees.

AA: Discussion regarding future potential elements (such as seating).

JF: An exciting project. Study another level of awning or vertical louvers to extend the shade larger than the footprint of the roof.

AA: Phase 2 of the project will potentially include additional shading elements and water features. Phase 1 will be limited to the pavilion element.

MS: Questions about court surface and drainage (AA: Concrete slab with a special paint. 0% Slope. Sloping is under consideration; to the south (½%).)

JV: Were there other considerations other than a basketball court? (Yes, many ideas have been considered; philanthropic support is making the court possible.) JV: Questions about potential or future water elements or features.

MS: Questions regarding the thickness of the canopy (AA: Approximately 15" at the edge, thicker towards the center.)

RM: Questions regarding lighting.

JF: Appreciates the curved roof form; references other round and curved structures in Hemisfair.

OVERALL COMMENTS:

Hemisfair Sports Court Pavilion

During numerous public meetings, focus groups and surveys, the citizens of San Antonio have expressed the need for community recreation in the planned Tower Park at Hemisfair. Therefore, through philanthropic support, we propose to construct a covered sports court where the pond is now. The pond, built in 1988 for the benefit of the Tower of the Americas operations, has a negative impact in the area for its unpleasant odor.

The proposed Sports Court is a covered basketball court with a simple but elegant design that takes its precedent from World Fair pavilions such as the IBM Pavilion from the 1968 World's Fair. The slender columns don't interrupt the views of the park, and the modified oval roof structure takes its precedent from the HemisFair '68 site plan. The inspiration for the resulting shape comes from the original Fair planning composition of circles and rectangles. Those elements consisted of the HemisFair Arena, the Tower of the Americas, the US Pavilion, and the Coca Cola Pavilion in the round category; and the Texas Pavilion and the Convention Center being large parallelogram objects at the opposite corners of the district.

The court will be a concrete slab 106'X 62' and striped as a 94' X 50' basketball court but able to be used as a multipurpose event space. The roof elevation will be at approximately 28 feet above the floor, with a flat soffit below the secondary beams. The roof slopes from the center, out to the perimeter at a 2% slope, and the metal covered fascia is two feet deep, giving the effect of a slender flat roof. The roof structure is generally oval in shape, supported by eight round steel columns supported by deep concrete piers. The roof overhangs the floor slab by ten feet at the midpoints of the sides and ends, providing shade for onlookers. The roof structure and piers are independent from the floor slab so that they can move separately.

At the east end of the court will be a metal covered plinth ten feet high which will house the electrical panel that will control the court's lighting and convenience outlets. There will also be a water fountain with multiple levels for ADA and pets.

Lighting for the pavilion will be mounted in several locations – up lights from the eight columns at floor level, up lights from the structure over the court, and downlights mounted to the inside of the major support beams.

The basketball backboard and rim will be mounted to the structure above and will be retractable with motorized controls.

In addition to sports activities, the public will be able to reserve the pavilion for a multitude of events.

Because this project is fully funded through philanthropic contributions, the plan is to start construction as soon as permitting is complete.

ISSUED SETS	
Date	Description

REVISIONS		
No.	Date	Description
01	05/01/2023	BIDDING SET



HEMISFAIR SPORTS COURT PAVILION. DESIGN DEVELOPMENT/BIDDING SET

Hemisfair Park, 434 S Alamo St.
San Antonio, Texas. 78205

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- GENERAL INFORMATION
- G0.01 COVER SHEET DRAWING INDEX
- A0.10 VICINITY & LOCATIONS
- A0.11 OVERALL SITE PLAN
- CIVIL
- C0.10 GENERAL CONSTRUCTION NOTES
- C0.20 STORM WATER POLLUTION PREVENTION PLAN
- C0.30 STORM WATER POLLUTION PREVENTION DETAILS
- C0.40 EXISTING CONDITIONS AND DEMOLITION PLAN
- C1.00 DIMENSIONAL CONTROL PLAN
- C2.00 GRADING PLAN
- C3.00 UTILITY PLAN
- C3.10 UTILITY DETAILS
- ARCHITECTURE
- A0.11 OVERALL SITE PLAN
- A1.01 FLOOR PLAN
- A1.02 ROOF PLAN
- A1.10 RCP
- A1.20 FINISH PLAN
- A2.01 ELEVATIONS
- A3.01 SECTIONS
- A6.10 FINISH SCHEDULE
- A7.01 DETAILS
- STRUCTURAL
- S0.00 NOTES
- S0.01 NOTES
- S0.02 ROOF CONNECTIONS AND DESIGN LOAD PLANS
- S1.00 DIMENSION CONTROL PLAN
- S1.01 FOUNDATION PLAN
- S1.10 ROOF PLAN
- S5.00 FOUNDATION DETAILS
- S5.10 STEEL FRAMING DETAILS
- ELECTRICAL
- E0.00 ELECTRICAL SYMBOLS AND ABBREVIATIONS
- E0.01 ELECTRICAL SITE PLAN
- E1.00 ELECTRICAL FLOOR PLAN
- E4.01 ELECTRICAL ONE LINE DIAGRAM
- E6.01 ELECTRICAL DETAILS

CONTACT LIST

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Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR
COORDINATION
ONLY

SHEET NAME
COVER SHEET

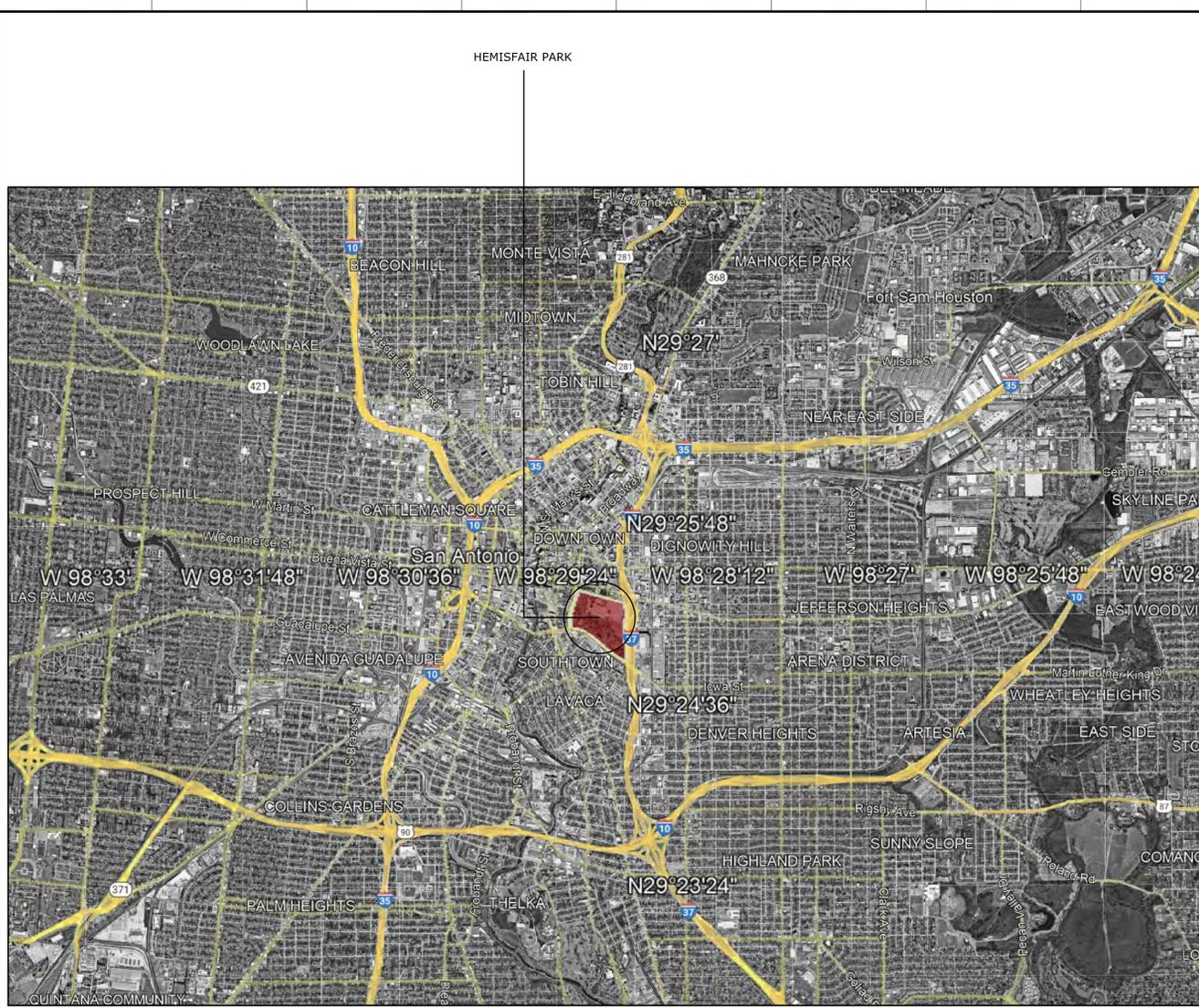
Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	05 / 01 / 2023	Code

SCALE
SHEET NUMBER

G001

NOT FOR CONSTRUCTION

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Date	Description

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR COORDINATION ONLY

SHEET NAME

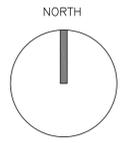
VICINITY & LOCATIONS/AREA SUMMARY

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	05 / 01 / 2023	Code

SCALE

SHEET NUMBER

A010



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DEMOLITION NOTES

- 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ALL NECESSARY PERMITS/APPROVALS BEFORE BEGINNING DEMOLITION.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING FROM THE SITE ALL ITEMS SHOWN TO BE DEMOLISHED UNLESS OTHERWISE INDICATED.
3. ALL EXISTING ITEMS NOT SPECIFICALLY NOTED TO BE DEMOLISHED SHALL REMAIN.
4. CONTRACTOR IS RESPONSIBLE FOR COORDINATION WITH ALL UTILITY COMPANIES REGARDING REMOVAL OF EXISTING SERVICES, POWER POLES TO BE REMOVED, VERIFYING UTILITIES ARE SHUT OFF OR DISCONNECTED, AND THAT ALL POSSIBLE SAFETY PRECAUTIONS HAVE BEEN ENACTED TO ENSURE THE SAFEST ENVIRONMENT FOR ALL PERSONNEL.
5. LOCATION AND DEPTH OF EXISTING UTILITIES SHOWN HEREON ARE APPROXIMATE ONLY. ACTUAL LOCATIONS AND DEPTHS MUST BE VERIFIED BY THE CONTRACTOR PRIOR TO THE CONSTRUCTION AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL UTILITIES, WHETHER SHOWN ON THE PLANS OR NOT, THROUGHOUT ALL PHASES OF CONSTRUCTION.
6. ALL NECESSARY EROSION CONTROL MEASURES ARE TO BE IN PLACE PRIOR TO CONSTRUCTION. EROSION CONTROL MEASURES ARE TO BE MAINTAINED AND IN WORKING CONDITION AT ALL TIMES.
7. CONTRACTOR SHALL CONFIRM WITH THE OWNER OR HIS DESIGNATE WHETHER TO SALVAGE AND MAKE ARRANGEMENTS TO STORE TRANSPLANTABLE TREES PRIOR TO REMOVAL.
8. FOR TREES SHOWN TO REMAIN, THE CONTRACTOR SHALL INSTALL TREE PROTECTION IN ACCORDANCE WITH THE PROJECT PLANS AND SPECIFICATIONS. THE CONTRACTOR SHALL NOT REMOVE OR DAMAGE ANY TREES WITHOUT A PERMIT TO DO SO.
9. NO PARKING AND/OR STORAGE SHALL BE ALLOWED WITHIN THE DRIP LINE OF THE TREES TO REMAIN.
10. THE CONTRACTOR SHALL SAW CUT EXISTING PAVEMENT, CURBS AND SIDEWALKS AT NEW PAVEMENT, CURB AND SIDEWALK JUNCTIONS, NO JAGGED OR IRREGULAR CUTS WILL BE ACCEPTED.
11. THE CONTRACTOR SHALL PROTECT ALL PROPERTY PINS, BENCH MARKS, CONSTRUCTION STAKES, HUBS, OR OTHER KEY CONTROL POINTS. THE CONTRACTOR SHALL BE RESPONSIBLE TO RE-ESTABLISH ANY SUCH POINTS AT THEIR OWN EXPENSE.
12. DEMOLITION CONTRACTOR IS RESPONSIBLE FOR CLEARING THE SITE OF ALL OBSTRUCTIONS THAT EXIST ON THIS SITE PRIOR TO THE START OF CONSTRUCTION OR DURING THE CONSTRUCTION SO AS TO NOT IMPEDE THE BUILDING CONSTRUCTION CONTRACTOR.
13. CONTRACTOR SHALL COORDINATE WITH THE OWNER TO IDENTIFY ANY MATERIAL OR EQUIPMENT SCHEDULED FOR REMOVAL TO BE SALVAGED AND REUSED. CONTRACTOR SHALL REPLACE AT HIS EXPENSE ANY DESTROYED MATERIAL OR EQUIPMENT THAT WAS MARKED FOR SALVAGE.
14. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL AND DISPOSAL OF ALL HAZARDOUS MATERIAL OFFSITE FOLLOWING ALL APPLICABLE DISPOSABLE REGULATIONS. ON SITE CONCRETE PROPOSED FOR DEMOLITION MAY BE REUSED ON SITE AS FILL AS LONG AS IT IS CRUSHED, FREE OF REBAR, WIRE MESH AND DEBRIS AND CAN MEET GEOTECHNICAL SPECIFICATIONS.
15. CONTRACTOR SHALL REMOVE ALL EXISTING IRRIGATION PIPING ON SITE UNLESS SHOWN OTHERWISE. CUT AND CAP LATERALS AT PROJECT LIMITS TO ALLOW PROPER FUNCTION OF ZONES INTENDED TO REMAIN OR EXTEND OFF-SITE.
16. CONTRACTOR SHALL NOT DEMOLISH ANY PUBLIC WATER OR SANITARY SEWER LINES WITHOUT APPROVAL. EXISTING WATER AND SANITARY SEWER SERVICES SHALL REMAIN OPERATIONAL UNTIL NEW SERVICE IS COMPLETE. CUT AND CAP ANY ABANDONED SANITARY SEWER AND WATER SERVICES AT THE EXISTING MAIN. NO ABANDONED SERVICES SHALL REMAIN CONNECTED TO THE PUBLIC MAIN.
17. THE USE OF EXPLOSIVES WILL NOT BE PERMITTED.
18. ALL WASTE MATERIAL REMAINING AFTER OWNER SALVAGE IS COMPLETE AND RESULTING FROM DEMOLITION OPERATIONS BECOMES THE PROPERTY OF THE CONTRACTOR. APPROPRIATE DISPOSAL OF WASTE MATERIAL SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AT HIS OWN EXPENSE. OWNER WILL PROVIDE LIST OF ITEMS TO BE SALVAGED.
19. THE CONTRACTOR SHALL MAINTAIN THE SITE IN A CLEAN AND ORDERLY MANNER.
20. THE CONTRACTOR SHALL MEET ALL LOCAL, STATE, AND FEDERAL REGULATIONS FOR DUST CONTROL. THE CONTRACTOR SHALL BE RESPONSIBLE AT THEIR OWN EXPENSE FOR ANY FUGITIVE DUST ON ADJOINING PROPERTIES.

DIMENSIONAL CONTROL NOTES

- 1. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY QUESTIONS THAT MAY ARISE CONCERNING THE INTENT, PLACEMENT OR LIMITS OF DIMENSIONS NECESSARY FOR CONSTRUCTION OF THE PROJECT.
2. THE CONTRACTOR SHALL PRESERVE ALL CONTROL POINTS, PROPERTY PINS, BENCH MARKS, HUBS OR OTHER KEY CONTROL POINTS. THE CONTRACTOR SHALL BE RESPONSIBLE TO RE-ESTABLISH ANY SUCH POINTS AT THEIR OWN EXPENSE IN THE EVENT THEY ARE REMOVED.
3. DIMENSIONAL CONTROL FOR ANY STRUCTURE IS BASED ON INFORMATION PROVIDED BY THE ARCHITECT OR STRUCTURAL ENGINEER. THE CONTRACTOR SHALL VERIFY ALL PROJECT DIMENSIONS WITH THE PROJECT DRAWINGS PRIOR TO CONSTRUCTION AND TO COMMUNICATE TO THE ENGINEER OF ANY DISCREPANCIES.
4. UNLESS OTHERWISE NOTED, THE CONTRACTOR SHALL USE THE TRAVERSE CONTROL POINTS FOR HORIZONTAL CONTROL POINTS. IF TRAVERSE CONTROL POINTS ARE NOT PROVIDED, THE CONTRACTOR MAY USE PROPERTY CORNER PINS, BENCHMARKS ARE NOT TO BE USED FOR HORIZONTAL CONTROL.
5. COORDINATES FOR HORIZONTAL CONTROL POINTS ARE BASED ON THE TEXAS STATE PLANE COORDINATE SYSTEM, SOUTH CENTRAL ZONE, AND 83(98) DISPLAYED IN SURFACE VALUES USING A SURFACE ADJUSTMENT FACTOR FOR EACH COUNTY. (THE SURFACE ADJUSTMENT FACTOR FOR BEXAR COUNTY IS 1.00017. OTHER COUNTIES WILL HAVE A DIFFERENT FACTOR; CHECK WITH THE SURVEYOR TO OBTAIN THE CORRECT SURFACE ADJUSTMENT FACTOR FOR PROJECTS LOCATED OUTSIDE OF BEXAR COUNTY.)
6. BENCHMARK ELEVATIONS ARE BASED ON NAVD 88, GEOID 03.
7. ALL DIMENSIONAL CONTROL POINTS OR DIMENSIONS ARE TO THE FACE OF CURB, FACE OF RETAINING WALL AT THE BOTTOM TOE OF SLOPE, AND CENTER OF PAINT STRIPING. ALL DIMENSIONS ARE PERPENDICULAR TO THE POINT OF REFERENCE.
8. CURB RADI ARE 3' UNLESS OTHERWISE NOTED ON THE DRAWINGS.
9. REFER TO THE ARCHITECTURAL, STRUCTURAL, AND LANDSCAPE PLANS AS APPLICABLE FOR ADDITIONAL DIMENSIONAL CONTROL INFORMATION.
10. THE CONTRACTOR SHALL RELY ON THE INFORMATION PROVIDED ON THE DESIGNED AND SEALED CONSTRUCTION DRAWINGS. SUBJECT TO A SIGNED RELEASE AGREEMENT, CAD FILES MAY BE OBTAINED FROM THE ENGINEER FOR THE CONVENIENCE AND USE OF THE CONTRACTOR.

GRADING NOTES

- 1. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THIS SCOPE OF WORK WHERE NOT SPECIFICALLY COVERED IN THE SPECIFICATIONS OR GEOTECHNICAL REPORT SHALL CONFORM TO ALL APPLICABLE CITY, COUNTY AND TxDOT STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION (LATEST EDITION).
2. SITE PREPARATION, GRADING, EXCAVATION AND FILL SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT GEOTECHNICAL REPORT AND SPECIFICATIONS.
3. ALL SELECT FILL MATERIAL PROVIDED SHALL BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACING AND COMPACTING.
4. ALL ELEVATIONS AND PROPOSED CONTOURS SHOWN ON THIS GRADING PLAN REFLECT FINISHED GRADES. THE THICKNESS OF PAVING, BASE, GRASS, TOPSOIL, AND MULCH MUST BE SUBTRACTED TO OBTAIN SUBGRADE ELEVATIONS.
5. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY QUESTIONS THAT MAY ARISE CONCERNING THE INTENT, PLACEMENT, OR LIMITS OF DIMENSIONS OR GRADES NECESSARY FOR CONSTRUCTION OF THIS PROJECT.
6. THE CONTRACTOR SHALL VERIFY THE SUITABILITY OF ALL EXISTING AND PROPOSED SITE CONDITIONS INCLUDING GRADES AND DIMENSIONS BEFORE COMMENCEMENT OF CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY OF ANY DISCREPANCIES.
7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ALL PERMITS, TESTS, APPROVALS AND ACCEPTANCES REQUIRED TO COMPLETE CONSTRUCTION OF THIS PROJECT.
8. THE CONTRACTOR SHALL REMOVE TOP SOIL, GRASS, ROOTS, DEBRIS, ETC. AND DISPOSE OFF SITE. THOSE MATERIALS NOT SUITABLE FOR EMBANKMENT AND TOPSOIL, CLEAN STRIPINGS AND TOPSOIL MAY BE STOCKPILED ON SITE FOR REUSE IN A LOCATION SPECIFIED BY THE OWNER.
9. THE SITE CONTRACTOR SHALL BE RESPONSIBLE FOR SITE STABILIZATION. ALL DISTURBED AREAS SHALL BE REVEGETATED IN ACCORDANCE WITH PROJECT SPECIFICATIONS AND TPDES/SWPPP REQUIREMENTS. REFERENCE THE LANDSCAPE ARCHITECT'S PLAN, IF APPLICABLE.
10. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS (EROSION CONTROL MEASURES) TO KEEP DRAINAGE AND SILT FROM WASHING ONTO ADJACENT PROPERTY, STREETS, OR DRAINAGE WAYS. CONTRACTOR SHALL IMMEDIATELY REMOVE SILT/DEBRIS WHICH WASHES OFFSITE OR INTO EXISTING STORM DRAIN SYSTEMS. (SEE SWPPP PLANS & TPDES BOOK).
11. THE CONTRACTOR SHALL OBTAIN GRADES SHOWN HEREON WITHIN +/- ONE-TENTH (0.10) FOOT.
12. IN PROPOSED PAVING AREAS, IT IS INTENDED THAT THE MINIMUM GRADE IS 1% ALL EARTHEN SLOPES SHALL BE A MAXIMUM OF 3:1 AND A MINIMUM OF 2.0% UNLESS OTHERWISE SHOWN.
13. ACCESSIBILITY: SIDEWALK CROSS SLOPE SHALL NOT EXCEED 2%. SIDEWALK LONGITUDINAL SLOPE ALONG ACCESSIBLE ROUTES SHALL NOT EXCEED 5%, UNLESS OTHERWISE NOTED. SIDEWALK CURB RAMPS SHALL NOT EXCEED 8.33% (SEE CURB RAMP DETAILS). CURB RAMP LANDINGS SHALL NOT EXCEED 2% ACCESSIBLE PARKING STALLS SHALL NOT EXCEED 2% IN ANY DIRECTION.
14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR RESTORING TO ITS ORIGINAL, OR BETTER, CONDITION ANY DAMAGE DONE TO EXISTING TREES, BUILDINGS, UTILITIES, FENCES, PAVEMENT, CURBS, OR DRIVEWAYS (NO SEPARATE PAY ITEMS).
15. THE CONTRACTOR SHALL EXERCISE EXTREME CAUTION IN WORKING NEAR UTILITIES, GAS LINES, SEWER, OR EXISTING APPURTENANCES. PRIOR TO PERFORMING ANY EXCAVATION, CONTRACTOR SHALL CONTACT ALL UTILITY COMPANIES AND ENSURE UTILITIES HAVE BEEN ADEQUATELY LOCATED AND IDENTIFIED. THE ENGINEER SHALL BE NOTIFIED IF ANY UTILITY CONFLICTS ARE DISCOVERED.
16. POSITIVE DRAINAGE SHALL BE MAINTAINED THROUGHOUT THE SCOPE OF THE PROJECT. DRAINAGE SHALL BE DIRECTED AWAY FROM ALL BUILDING FOUNDATIONS. CONTRACTOR SHOULD TAKE PRECAUTIONS NOT TO ALLOW ANY PONDING OF WATER.
17. FOR FILL PLACEMENT ON HILL SIDES OR STEEP SLOPE AREAS, THE CONTRACTOR SHALL REFERENCE THE PROJECT SPECIFICATIONS AND GEOTECHNICAL REPORT FOR SPECIAL INSTRUCTIONS REGARDING BENCHING.
18. NO WORK SHALL BE PERFORMED IN A PUBLIC RIGHT-OF-WAY WITHOUT A PERMIT.

SITE UTILITY NOTES

- 1. THE CONTRACTOR WILL BE RESPONSIBLE FOR DETERMINING EXACT LOCATION OF ALL UTILITIES AND DRAINAGE STRUCTURES WHETHER SHOWN ON THE PLANS OR NOT. THE CONTRACTOR SHALL UNCOVER EXISTING UTILITIES PRIOR TO CONSTRUCTION TO VERIFY SIZE, GRADE, AND LOCATION. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY OF ANY DEVIATIONS FROM PLANS PRIOR TO BEGINNING CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES, WHETHER SHOWN ON THE PLANS OR NOT, SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO REPAIR, AT HIS EXPENSE.
2. DRAWINGS DO NOT SHOW ALL EXISTING UTILITIES. ALL EXISTING UTILITIES SHALL BE VERIFIED IN THE FIELD WHETHER SHOWN ON THIS PLAN OR NOT (PRIOR TO INSTALLATION OF ANY NEW LINES).
3. ALL FILL MATERIAL IS TO BE IN PLACE AND COMPACTED BEFORE INSTALLATION OF PROPOSED UTILITIES.
4. CONTRACTOR SHALL CALL FOR THE LOCAL JURISDICTIONAL INSPECTIONS AT LEAST 48 HOURS PRIOR TO STARTING CONSTRUCTION.
5. CONTRACTOR IS RESPONSIBLE FOR COMPLYING TO THE SPECIFICATIONS OF THE LOCAL JURISDICTION WITH REGARDS TO MATERIALS AND INSTALLATION OF THE UTILITIES AND STORM DRAINS.
6. CONTRACTOR SHALL COORDINATE WITH ALL UTILITY COMPANIES FOR INSTALLATION REQUIREMENTS, SPECIFICATIONS AND ALL TESTING.
7. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THE SCOPE OF THIS PROJECT SHALL COMPLY WITH THE FOLLOWING AS APPLICABLE:
A. CURRENT "SAN ANTONIO WATER SYSTEM STANDARD SPECIFICATIONS FOR CONSTRUCTION"
B. CURRENT "SAN ANTONIO WATER SYSTEM UTILITY SERVICE REGULATIONS"
C. CURRENT CITY OF SAN ANTONIO "STANDARD SPECIFICATIONS FOR PUBLIC WORKS CONSTRUCTION"
D. CURRENT TxDOT "STANDARD SPECIFICATIONS FOR CONSTRUCTION OF HIGHWAYS, STREETS, AND DRAINAGE"
E. CURRENT CITY OF SAN ANTONIO "RIGHT-OF-WAY ORDINANCE AND CRITERIA MANUAL"
8. MINIMUM TRENCH WIDTH SHALL BE 2 FEET.
9. ALL CONCRETE FOR ENCASEMENTS SHALL HAVE A MINIMUM 28 DAY COMPRESSION STRENGTH AT 3000 P.S.I.
10. CONTRACTOR SHALL PROTECT ALL EXISTING TREES, FENCES, PAVING, UTILITIES, AND OTHER STRUCTURES SCHEDULED TO REMAIN. ANY STRUCTURE DAMAGED BY THE CONTRACTOR SHALL BE REPAIRED BY THE CONTRACTOR AT THEIR EXPENSE.
11. THE CONTRACTOR SHALL FURNISH THE ENGINEER WITH ALL FINAL UTILITY AS-BUILT MEASUREMENTS, TOPS AND LENGTH OF SERVICE CONNECTIONS OF THE PROJECT.
12. ALL GARBAGE OR SPOIL MATERIAL FROM THIS WORK SHALL BE REMOVED FROM THE SITE BY THE CONTRACTOR AT HIS SOLE EXPENSE.
13. GAS AND ELECTRIC ALIGNMENTS SHOWN ON THIS DRAWING ARE CONCEPTUAL. THE ACTUAL DESIGN AND LOCATIONS SHALL BE DETERMINED BY THE LOCAL SERVICE PROVIDER OR MEP ENGINEER.
14. CONTRACTOR SHALL COORDINATE TELE. COMMUNICATIONS, CABLE, ELECTRIC AND GAS LINE INSTALLATION WITH LOCAL SERVICE PROVIDER. THE SERVICE PROVIDER WILL BE RESPONSIBLE FOR INSTALLATION OF GAS LINE TO WITHIN 5' OF BUILDING.
15. REFER TO INTERIOR PLUMBING DRAWINGS FOR TIE-IN OF ALL UTILITIES.
16. SEE IRRIGATION, LIGHTING AND ARCHITECTURAL PLANS FOR ADDITIONAL CONDUIT LOCATIONS AS APPLICABLE. VERIFY ALL CONDUIT AND SLEEVE LOCATIONS PRIOR TO PLACING ANY PAVEMENT.
17. CONTRACTOR SHALL INSTALL ALL CONDUITS WITH A MINIMUM 4-FOOT SWEEP RADIUS. ALL CONDUITS SHALL HAVE A PULL STRING TO BE INSTALLED BY THE CONTRACTOR.
18. NO WORK SHALL BE ALLOWED WITHIN THE PUBLIC RIGHT-OF-WAY WITHOUT AN APPROVED PERMIT.
19. THE CONSTRUCTION OF UNDERGROUND PRIMARY ELECTRIC AND GAS DISTRIBUTION SYSTEMS SHALL BE GOVERNED BY THE ENGINEERING CONSTRUCTION PLANS PREPARED BY THE LOCAL SERVICE PROVIDER. THIS DRAWING SHALL SERVE ONLY AS REFERENCE DOCUMENT TO COORDINATE LOCATION OF THE PROPOSED PRIMARY ELECTRIC AND GAS DISTRIBUTION SYSTEM. THE LOCAL SERVICE PROVIDER'S CONSTRUCTION DRAWINGS AND CONSTRUCTION DETAILS SHALL GOVERN.
20. CONTRACTOR SHALL INCLUDE IN HIS BID A 4" PVC CONDUIT FOR TELEPHONE AND A 2" PVC CONDUIT FOR CABLE TV TO BE IN THE SAME TRENCH AS UNDERGROUND ELECTRIC LINES. CONTRACTOR SHALL VERIFY WITH APPROPRIATE UTILITY COMPANY PRIOR TO CONSTRUCTION ON NUMBER AND SIZE OF CONDUITS NEEDED FOR UTILITY SERVICE TO ALL BUILDINGS.
21. BEDDING FOR ALL UTILITIES SHALL BE PER THE PROJECT SPECIFICATIONS. NO WATER JETTING OF BACKFILL MATERIAL WILL BE ALLOWED.

DRAINAGE NOTES

- 1. ALL MATERIALS AND CONSTRUCTION PROCEDURES WITHIN THIS SCOPE OF WORK SHALL COMPLY WITH THE PROJECT GEOTECH REPORT, THE PROJECT SPECIFICATIONS, AND THE CURRENT CITY, COUNTY OR TxDOT.
2. THE CONTRACTOR SHALL PROTECT ALL EXISTING UTILITIES. THE CONTRACTOR SHOULD EXERCISE EXTREME CAUTION WHEN WORKING NEAR EXISTING UTILITIES AND SHOULD THEY BE DAMAGED DURING CONSTRUCTION OPERATIONS, THE CONTRACTOR WILL BE REQUIRED TO REPAIR OR REPLACE THE DAMAGED FACILITIES AT CONTRACTOR'S EXPENSE.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR RESTORING TO ORIGINAL OR BETTER CONDITION DAMAGE DONE TO EXISTING FENCES, CURBS, STREETS, DRIVEWAYS, LANDSCAPING AND STRUCTURES.
4. CONTRACTOR IS RESPONSIBLE FOR REMOVAL OF ALL WASTE MATERIALS UPON PROJECT COMPLETION.
5. WATER JETTING THE BACKFILL OF STORM DRAIN TRENCHES WILL NOT BE PERMITTED.
6. NORTHINGS AND EASTINGS LISTED ON THESE PLANS ARE TO CENTER OF BOX FOR JUNCTION BOXES AND GRATE INLETS AND TO OUTSIDE CORNER FACE OF CURB FOR ALL CURB AND COMBINATION INLETS. ALL LENGTHS OF PIPE ARE TO INSIDE FACE OF STRUCTURES.
7. CONTRACTOR SHALL ENSURE PROPER SIZE OF JUNCTION BOXES NEEDED WHERE INDICATED ON PLAN. CONTRACTOR SHALL CONNECT STORM DRAIN PIPE TO JUNCTION BOXES PER MANUFACTURERS SPECIFICATIONS.
8. ALL STORM DRAIN TO JUNCTION BOX CONNECTIONS SHALL HAVE CONCRETE COLLARS.
9. ALL GRATE INLETS MUST BE HS20 EQUIVALENT RATED GRATES.
10. TOPS OF MANHOLES, JUNCTION BOXES AND GRATES SHALL BE SET FLUSH TO FINISHED SURFACE BASED UPON GRADING PLAN.
11. CONTRACTOR SHALL GROUT INVERTS OF ALL STORM DRAIN INLETS, JUNCTION BOXES, AND DROP STRUCTURES TO DRAIN.

CAUTION UNDERGROUND UTILITIES

THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR SHALL BE REQUIRED TO LOCATE ALL PUBLIC OR PRIVATE UTILITIES INCLUDING BUT NOT LIMITED TO: WATER, SEWER, TELEPHONE, AND FIBER OPTIC LINES, SITE LIGHTING ELECTRIC, SECONDARY ELECTRIC, PRIMARY ELECTRICAL DUCT BANKS, LANDSCAPE IRRIGATION FACILITIES, AND GAS LINES. THE CONTRACTOR MUST CONTACT 1-800-DIG-TESS AND CALL THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION AND/OR START OF CONSTRUCTION TO REQUEST EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES (WHETHER SHOWN ON PLANS OR NOT) WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS. ANY UTILITY CONFLICTS THAT ARISE SHOULD BE COMMUNICATED TO THE ENGINEER IMMEDIATELY AND PRIOR TO CONSTRUCTION. ANY DAMAGE TO EXISTING UTILITIES SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR AND THE REPAIR SHALL BE AT CONTRACTORS SOLE EXPENSE WHETHER THE UTILITY IS SHOWN ON THESE PLANS OR NOT.

CAUTION OVERHEAD UTILITIES

CONTRACTOR TO EXERCISE EXTREME CAUTION WHEN WORKING UNDER "HIGH VOLTAGE TRANSMISSION LINES". A WORKING HEIGHT OF 30' FROM GROUND ELEVATION WILL BE OBSERVED WHEN WORKING UNDER THE HIGH VOLTAGE LINE. COORDINATE ALL WORK WITH THE LOCAL UTILITY PROVIDER.

TRENCH EXCAVATION SAFETY PROTECTION

CONTRACTOR AND/ OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR STRUCTURAL DESIGN/ GEOTECHNICAL/ SAFETY/EQUIPMENT CONSULTANT, IF ANY, SHALL REVIEW THESE PLANS AND ANY AVAILABLE GEOTECHNICAL INFORMATION AND THE ANTICIPATED INSTALLATION SITES WITHIN THE PROJECT WORK AREA IN ORDER TO IMPLEMENT CONTRACTOR'S TRENCH EXCAVATION SAFETY PROTECTION SYSTEMS, PROGRAMS AND /OR PROCEDURES FOR THE PROJECT DESCRIBED IN THE CONTRACT DOCUMENTS. THE CONTRACTOR'S IMPLEMENTATION OF THESE SYSTEMS, PROGRAMS AND/OR PROCEDURES SHALL PROVIDE FOR ADEQUATE TRENCH EXCAVATION SAFETY PROTECTION THAT COMPLY WITH AS A MINIMUM, OSHA STANDARDS FOR TRENCH EXCAVATIONS. SPECIFICALLY, CONTRACTOR AND/OR CONTRACTOR'S INDEPENDENTLY RETAINED EMPLOYEE OR SAFETY CONSULTANT SHALL IMPLEMENT A TRENCH SAFETY PROGRAM IN ACCORDANCE WITH OSHA STANDARDS GOVERNING THE PRESENCE AND ACTIVITIES OF INDIVIDUALS WORKING IN AND AROUND TRENCH EXCAVATION.

Table with 2 columns: Date, Description

Table with 3 columns: No., Date, Description



Hemisfair Sports Court Pavilion
Hemisfair San Antonio, TX. 78205

2000 HW LOOP 410 | SAN ANTONIO, TX 78213 | 210.375.9000
TEXAS ENGINEERING FIRM #470 | TEXAS SURVEYING FIRM #140328800

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Table with 2 columns: Draftsman, Phase

BID SET

C0.10

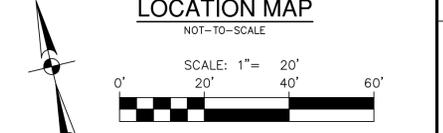
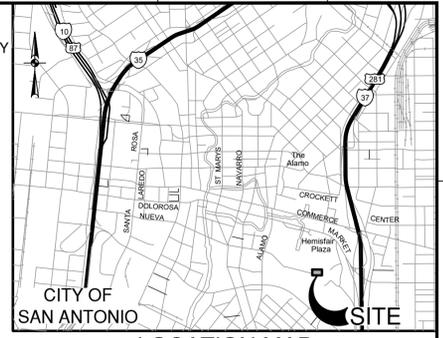
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SWP3 MODIFICATIONS		
DATE	SIGNATURE	DESCRIPTION

ADDRESS
 600 HEMISFAIR PLAZA WAY
 SAN ANTONIO, TX 78214

LEGAL DESCRIPTION
 LOT 12 & 903
 BLOCK 3
 N.C.B. 13814

PLAT
 160546



LEGEND

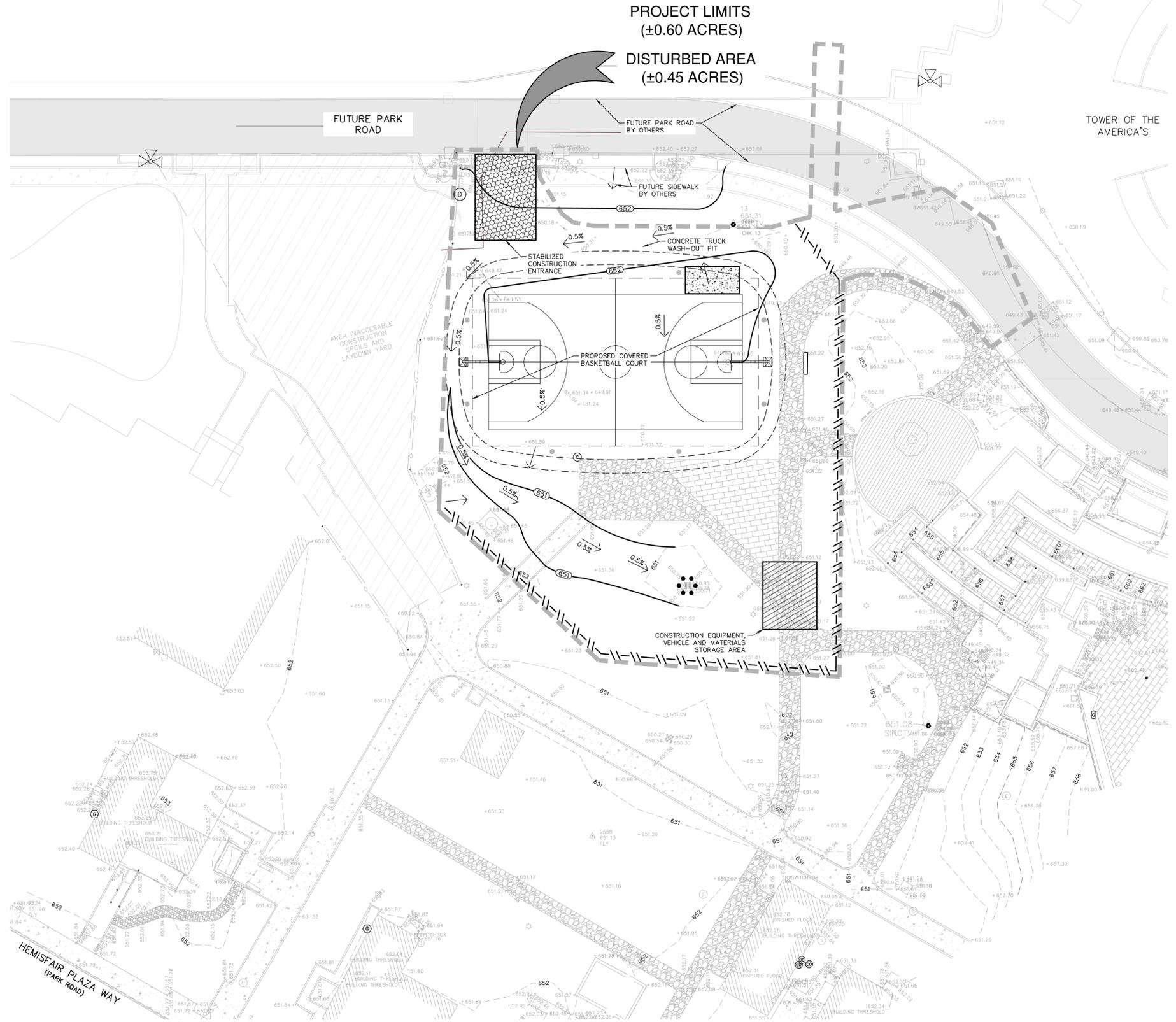
	PROJECT LIMITS (0.44 AC)
	EXISTING CONTOUR
	PROPOSED CONTOUR
	FLOW ARROW (EXISTING)
	FLOW ARROW (PROPOSED)
	SILT FENCE
	GRATE INLET PROTECTION
	STABILIZED CONSTRUCTION ENTRANCE/EXIT (FIELD LOCATE)
	CONSTRUCTION EQUIPMENT, VEHICLE & MATERIALS STORAGE AREA (FIELD LOCATE)
	CONCRETE TRUCK WASH-OUT PIT (FIELD LOCATE)

- GENERAL NOTES**
- DO NOT DISTURB VEGETATED AREAS (TREES, GRASS, WEEDS, BRUSH, ETC.) ANY MORE THAN NECESSARY FOR CONSTRUCTION.
 - CONSTRUCTION ENTRANCE/EXIT LOCATION, CONCRETE WASH-OUT PIT, AND CONSTRUCTION EQUIPMENT AND MATERIAL STORAGE YARD TO BE DETERMINED IN THE FIELD.
 - STORM WATER POLLUTION PREVENTION CONTROLS MAY NEED TO BE MODIFIED IN THE FIELD TO ACCOMPLISH THE DESIRED EFFECT. ALL MODIFICATIONS ARE TO BE NOTED ON THIS EXHIBIT AND SIGNED AND DATED BY THE RESPONSIBLE PARTY.
 - RESTRICT ENTRY/EXIT TO THE PROJECT SITE TO DESIGNATED LOCATIONS BY USE OF ADEQUATE FENCING, IF NECESSARY.
 - ALL STORM WATER POLLUTION PREVENTION CONTROLS ARE TO BE MAINTAINED AND IN WORKING CONDITIONS AT ALL TIMES.
 - FOR A COMPLETE LISTING OF TEMPORARY STORM WATER POLLUTION PREVENTION PLAN.
 - STORM WATER POLLUTION PREVENTION STRUCTURES SHOULD BE CONSTRUCTED WITHIN THE SITE BOUNDARIES. SOME OF THESE FEATURES MAY BE SHOWN OUTSIDE THE SITE BOUNDARIES ON THIS PLAN FOR VISUAL CLARITY.
 - AS SOON AS PRACTICAL, ALL DISTURBED SOIL THAT WILL NOT BE COVERED BY IMPERVIOUS COVER SUCH AS PARKWAY AREAS, EASEMENT (VOL. 9, PG 285) AREAS, EMBANKMENT SLOPES, ETC. WILL BE STABILIZED PER APPLICABLE PROJECT SPECIFICATIONS.
 - BEST MANAGEMENT PRACTICES MAY BE INSTALLED IN STAGES TO COINCIDE WITH THE DISTURBANCE OF UPGRADIENT AREAS.
 - BEST MANAGEMENT PRACTICES MAY BE REMOVED IN STAGES ONCE THE WATERSHED FOR THAT PORTION CONTROLLED BY THE BEST MANAGEMENT PRACTICES HAS BEEN STABILIZED IN ACCORDANCE WITH TPDES REQUIREMENTS.
 - UPON COMPLETION OF THE PROJECT, INCLUDING SITE STABILIZATION, AND BEFORE FINAL PAYMENT IS ISSUED, CONTRACTOR SHALL REMOVE ALL SEDIMENT AND EROSION CONTROL MEASURES, PAYING SPECIAL ATTENTION TO ROCK BERMS IN DRAINAGE FEATURES.
 - WHERE VEGETATED FILTER STRIPS ARE INDICATED, CONTRACTOR SHALL VERIFY THAT SUFFICIENT VEGETATION EXISTS. OTHERWISE CONTRACTOR SHALL PLACE SILT FENCING IN LIEU OF VEGETATED FILTER STRIP.
 - SHADED AREA DENOTES LIMITS OF DISTURBED AREAS. OTHER AREAS WITHIN THE PROJECT LIMITS, WITH THE EXCEPTION OF A CONSTRUCTION EQUIPMENT AND MATERIAL STORAGE YARD, ARE NOT A PART OF THIS TPDES STORM WATER POLLUTION PREVENTION PLAN (SWP3) AND WILL NOT BE DISTURBED BY CIVIL CONSTRUCTION ACTIVITIES. HOUSE CONSTRUCTION ACTIVITIES WILL REQUIRE A SEPARATE STORM WATER POLLUTION PREVENTION PLAN.
 - PRIOR TO BEGINNING CONSTRUCTION, CONTRACTOR SHALL COORDINATE PLACEMENT OF TEMPORARY BEST MANAGEMENT PRACTICES WITHIN TxDOT RIGHT-OF-WAY WITH TxDOT.
 - CPS ENERGY MAY FUNCTION AS A SECONDARY OPERATOR ON THIS PROJECT AND MAY BE INSTALLING ELECTRIC UTILITIES FOR ON-SITE CONSTRUCTION AND OFF-SITE FEED TO THE PROJECT.

THE ENGINEERING SEAL HAS BEEN AFFIXED TO THIS SHEET ONLY FOR THE PURPOSE OF DEMONSTRATING COMPLIANCE WITH THE TPDES-STORM WATER POLLUTION PREVENTION PLAN (SWP3) REGULATIONS.

 THIS SHEET HAS BEEN PREPARED FOR PURPOSES OF THE SWP3 ONLY. ALL OTHER CIVIL ENGINEERING RELATED INFORMATION SHOULD BE ACQUIRED FROM THE APPROPRIATE SHEET IN THE CIVIL IMPROVEMENT PLANS.

EXHIBIT 2



PROJECT LIMITS
 (±0.60 ACRES)

DISTURBED AREA
 (±0.45 ACRES)

FUTURE PARK ROAD

FUTURE PARK ROAD BY OTHERS

FUTURE SIDEWALK BY OTHERS

STABILIZED CONSTRUCTION ENTRANCE

CONCRETE TRUCK WASH-OUT PIT

PROPOSED COVERED BASKETBALL COURT

CONSTRUCTION EQUIPMENT, VEHICLE AND MATERIALS STORAGE AREA

TOWER OF THE AMERICA'S

HEMISFAIR PLAZA WAY (PARK ROAD)

BID SET

GVI GOMEZ VAZQUEZ INTERNATIONAL

ARCHITECTURE | PLANNING | URBAN DESIGN

210-404-9658 www.gvi.archi

ISSUED SETS

Date	Description

REVISIONS

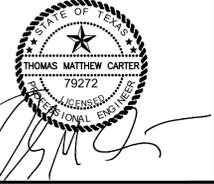
No.	Date	Description
01	05/01/2023	BIDDING SET

PAPE-DAWSON ENGINEERS

Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

5/1/2023



SHEET NAME

STORM WATER POLLUTION PREVENTION PLAN

Draftsman: **gs** Phase: **BIDDING SET**

GVI Approval: Project No. **7645-56**

Client Approval: File

Date: **MAY 2023** Code

SCALE

SHEET NUMBER

C0.20

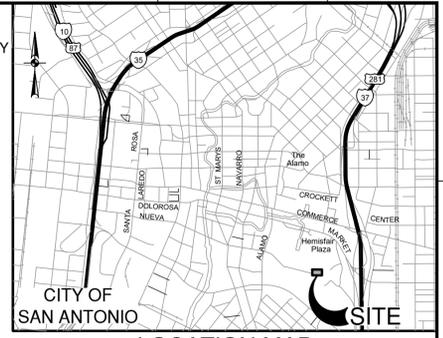
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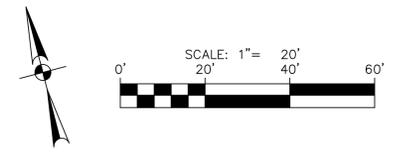
ADDRESS
 600 HEMISFAIR PLAZA WAY
 SAN ANTONIO, TX 78214

LEGAL DESCRIPTION
 LOT 12 & 903
 BLOCK 3
 N.C.B. 13814

PLAT
 160546



LOCATION MAP
 NOT-TO-SCALE

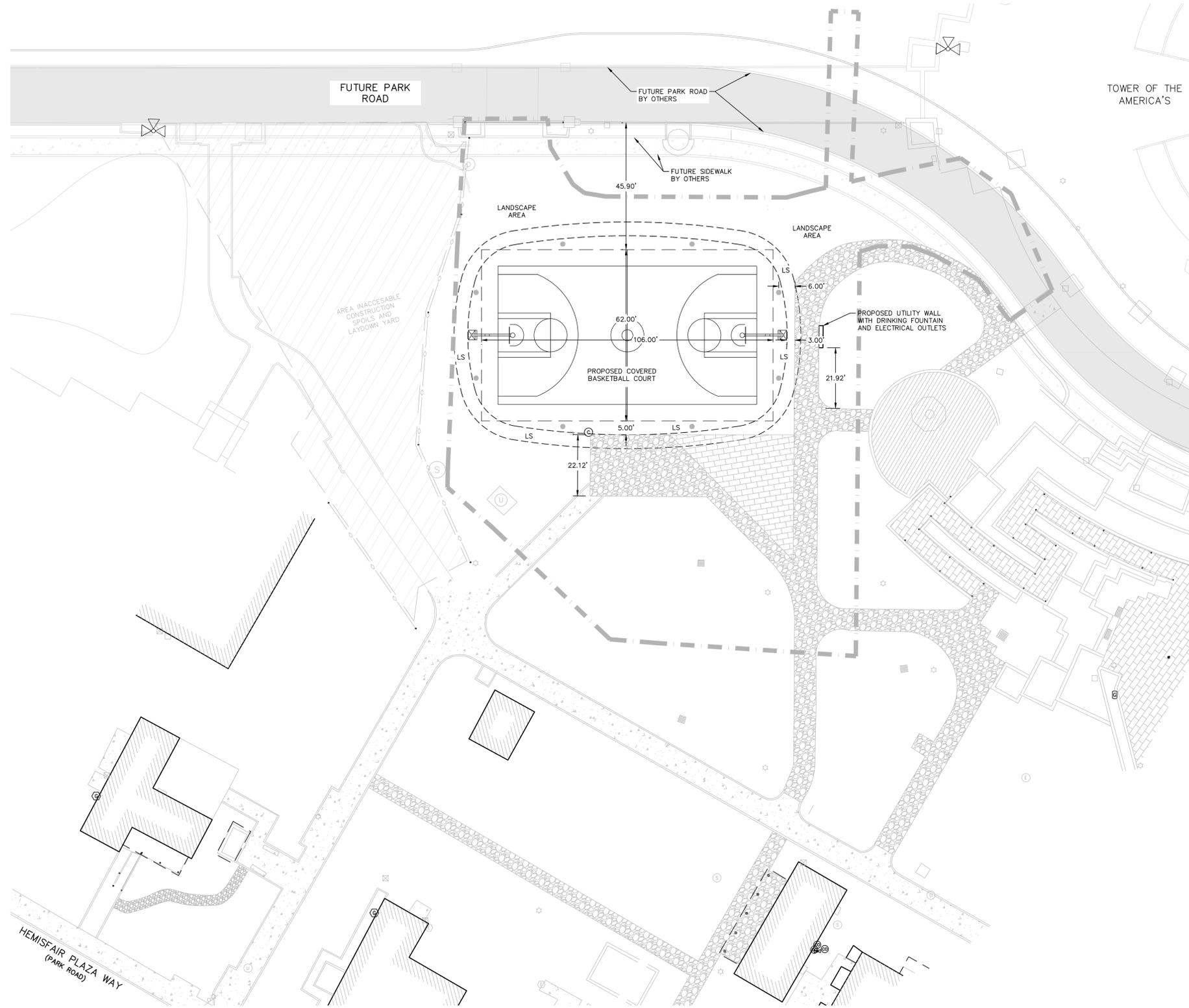


LEGEND

- PROPERTY LINE
- LIMITS OF CONSTRUCTION
- LANDSCAPE AREA

NOTE

REFERENCE SHEET C0.10 FOR GENERAL CONSTRUCTION NOTES.



BID SET

GVI GOMEZ VAZQUEZ INTERNATIONAL

ARCHITECTURE | PLANNING | URBAN DESIGN
 210-404-9658 www.gvi.archi

ISSUED SETS

Date	Description

REVISIONS

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PAPE-DAWSON ENGINEERS

2000 NW LOOP 410 | SAN ANTONIO, TX 78213 | 210.375.9000
 TEXAS ENGINEERING FIRM #470 | TEXAS SURVEYING FIRM #10028800

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PROFESSIONAL SEAL

5/1/2023

THOMAS MATTHEW CARTER
 79272
 PROFESSIONAL ENGINEER

SHEET NAME

DIMENSIONAL CONTROL PLAN

Draftsman	es	Phase	BIDDING SET
GVI Approval		Project No.	7645-56
Client Approval		File	
Date	MAY 2023	Code	

SCALE

SHEET NUMBER

C1.00

ISSUED SETS

Date	Description

REVISIONS

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR COORDINATION ONLY

SHEET NAME

SITE PLAN

Draftsman _____ Phase DESIGN DEVELOPMENT

GVI Approval _____ Project No. 22118

Client Approval _____ File _____

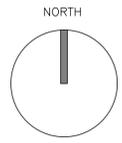
Date 05 / 01 / 2023 Code _____

SCALE _____

SHEET NUMBER

A011

1 Overall Site Plan
SCALE: 1/16" = 1'-0"



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Date	Description

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion

Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR COORDINATION ONLY

SHEET NAME

COURT FLOOR PLAN

Draftsman Phase DESIGN DEVELOPMENT

GVI Approval Project No. 22118

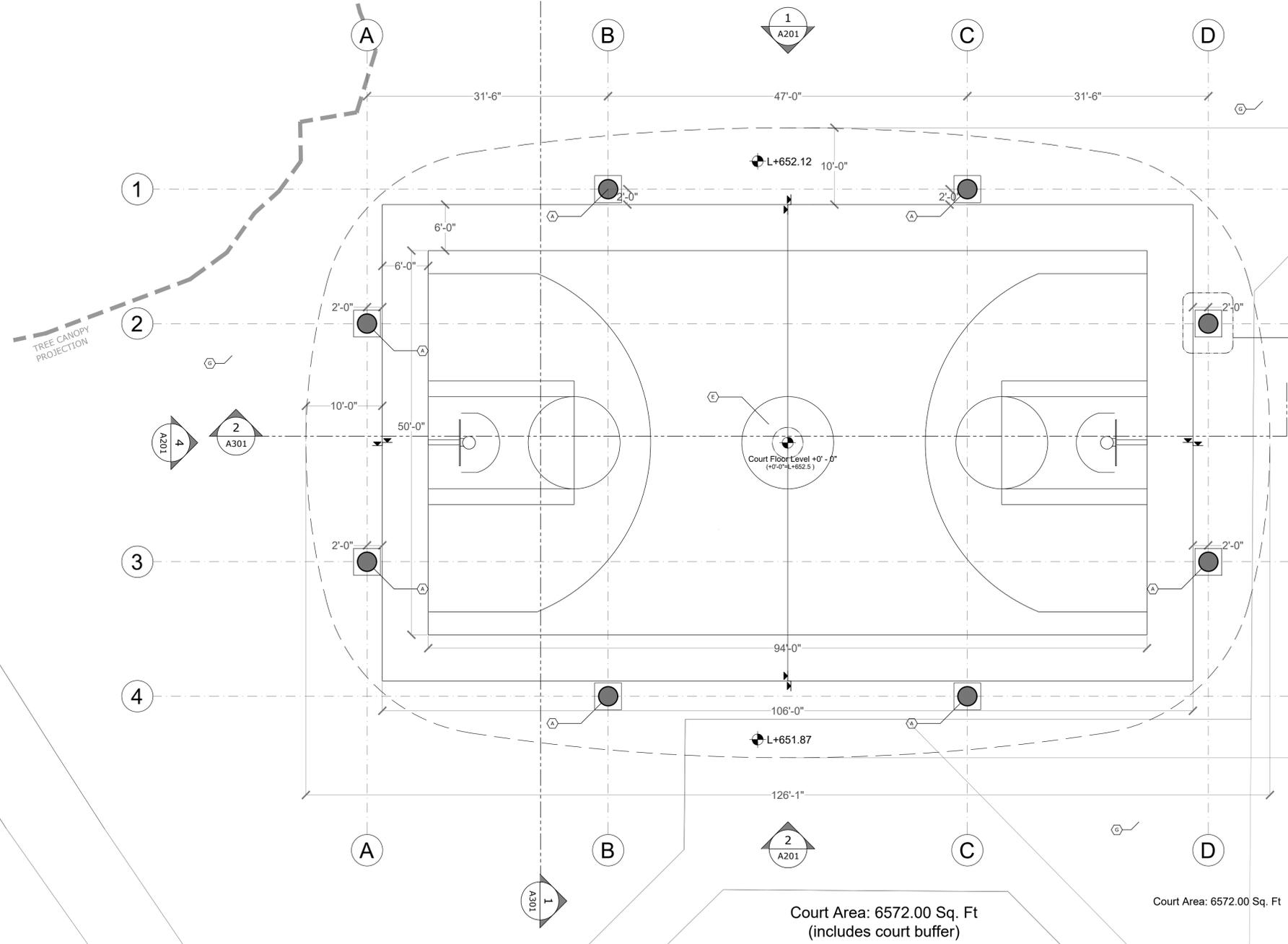
Client Approval File

Date 05 / 01 / 2023 Code

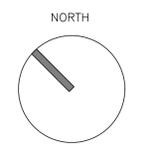
SCALE

SHEET NUMBER

A101



1 Floor Plan
SCALE: 1/8" = 1'-0"



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ISSUED SETS

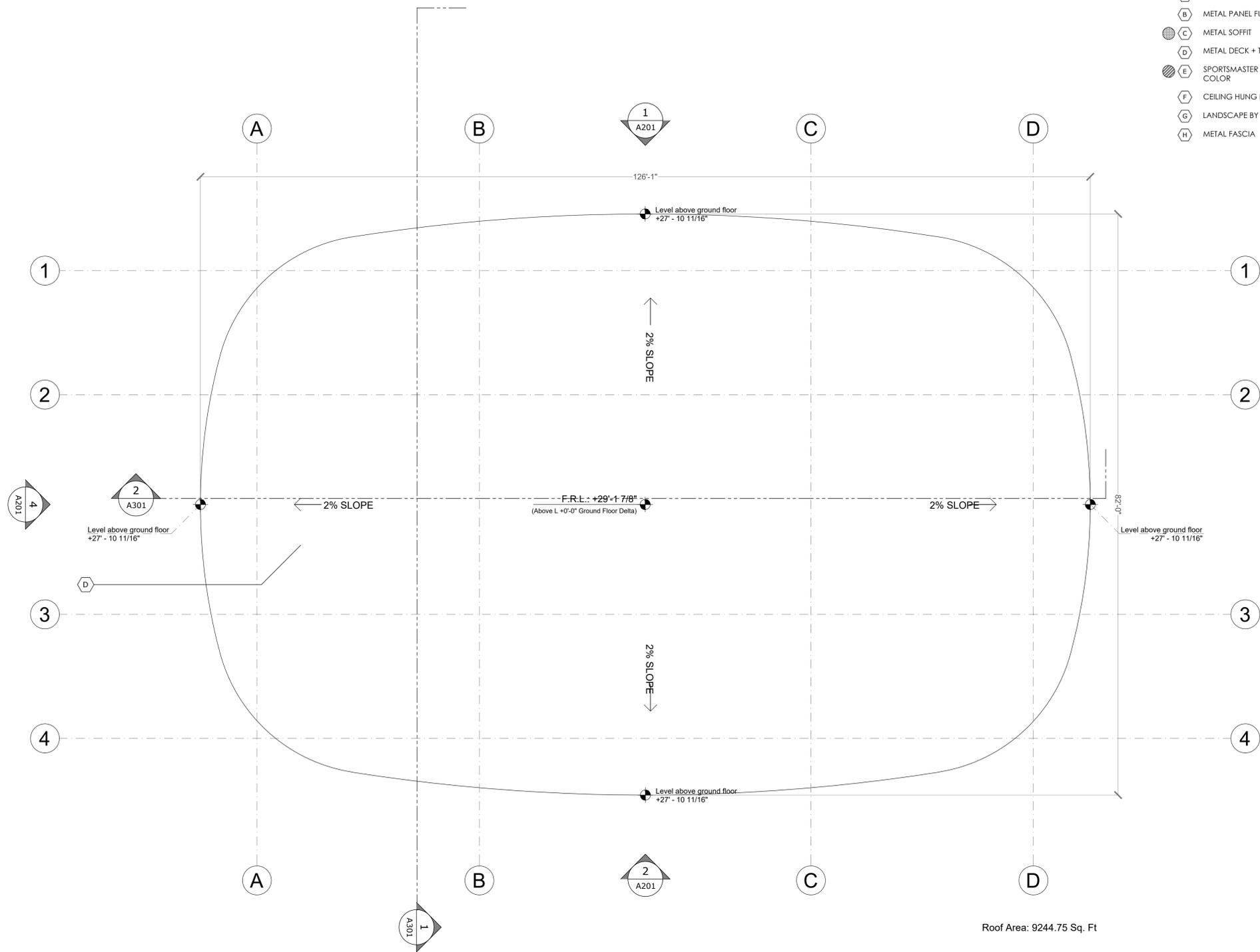
Date	Description

REVISIONS

No.	Date	Description
01	05/01/2023	BIDDING SET

KEY NOTES

- (A) METAL COLUMN-PROTECTIVE ENAMEL FLAT EXTERIOR PAINT
- (B) METAL PANEL FURROUT
- (C) METAL SOFFIT
- (D) METAL DECK + TPO
- (E) SPORTSMaster COLORPLUS SYSTEM ON CONCRETE COLOR
- (F) CEILING HUNG BACKSTOP
- (G) LANDSCAPE BY OTHERS (NOT IN SCOPE)
- (H) METAL FASCIA



1 Roof Plan
SCALE: 1/8" = 1'-0"



NOT FOR CONSTRUCTION

Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR COORDINATION ONLY

SHEET NAME
ROOF PLAN

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	Code	05 / 01 / 2023

SCALE
SHEET NUMBER
A102

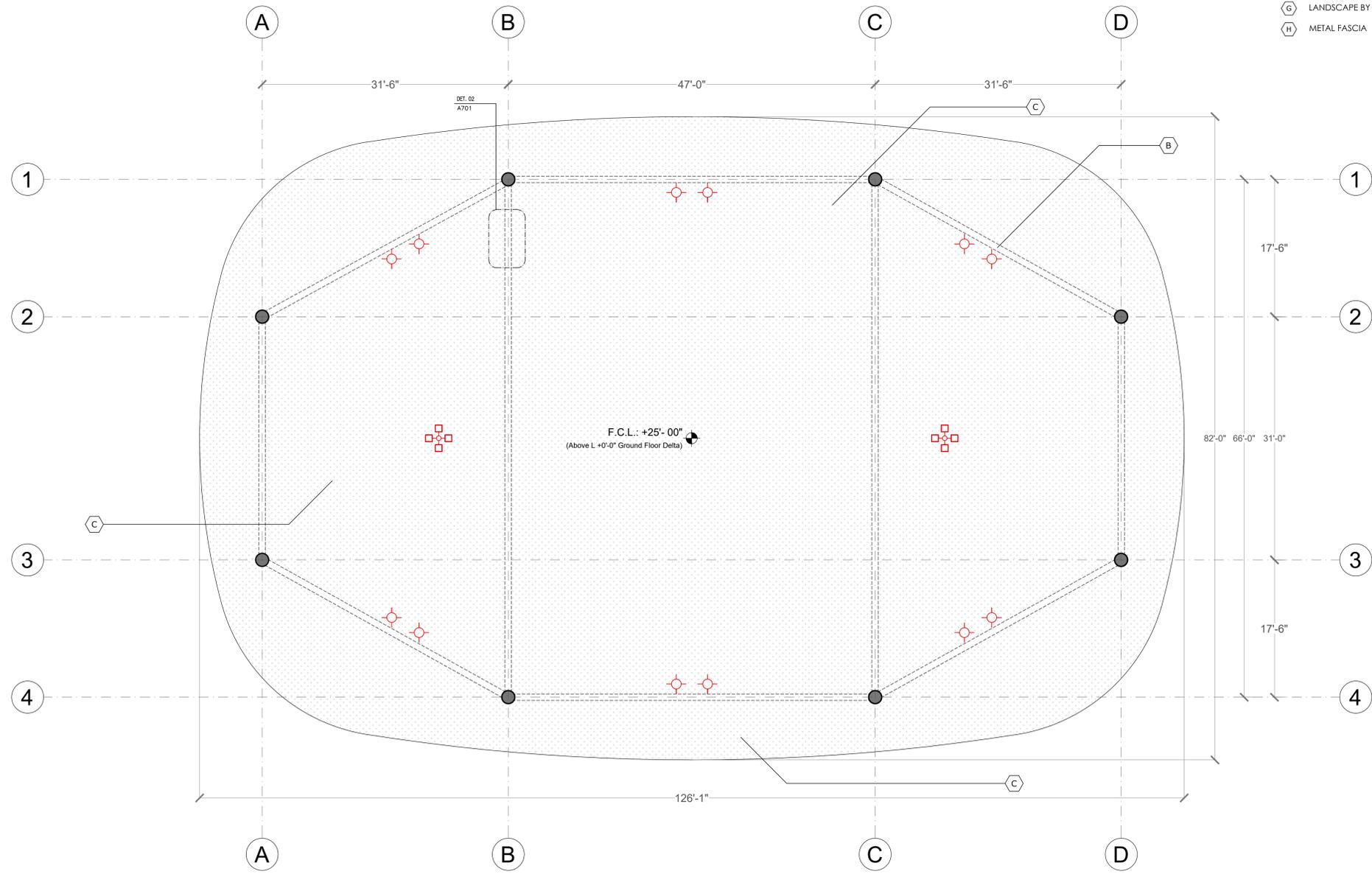
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Date	Description

No.	Date	Description
01	05/01/2023	BIDDING SET

KEY NOTES

- (A) METAL COLUMN-PROTECTIVE ENAMEL FLAT EXTERIOR PAINT
- (B) METAL PANEL FURROUT
- (C) METAL SOFFIT
- (D) METAL DECK + TPO
- (E) SPORTSMASTER COLORPLUS SYSTEM ON CONCRETE COLOR
- (F) CEILING HUNG BACKSTOP
- (G) LANDSCAPE BY OTHERS (NOT IN SCOPE)
- (H) METAL FASCIA



Soffit Area: 9244.75 Sq. Ft
 RE: ELECTRICAL AND LIGHTING DESIGNER
 PRESENTATION FOR LIGHTING FIXTURES

1 Reflected Ceiling Plan
 SCALE: 1/8" = 1'-0"



NOT FOR CONSTRUCTION

Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR COORDINATION ONLY

SHEET NAME

REFLECTED CEILING PLAN

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	Code	05 / 01 / 2023

SCALE

SHEET NUMBER

A110

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ISSUED SETS

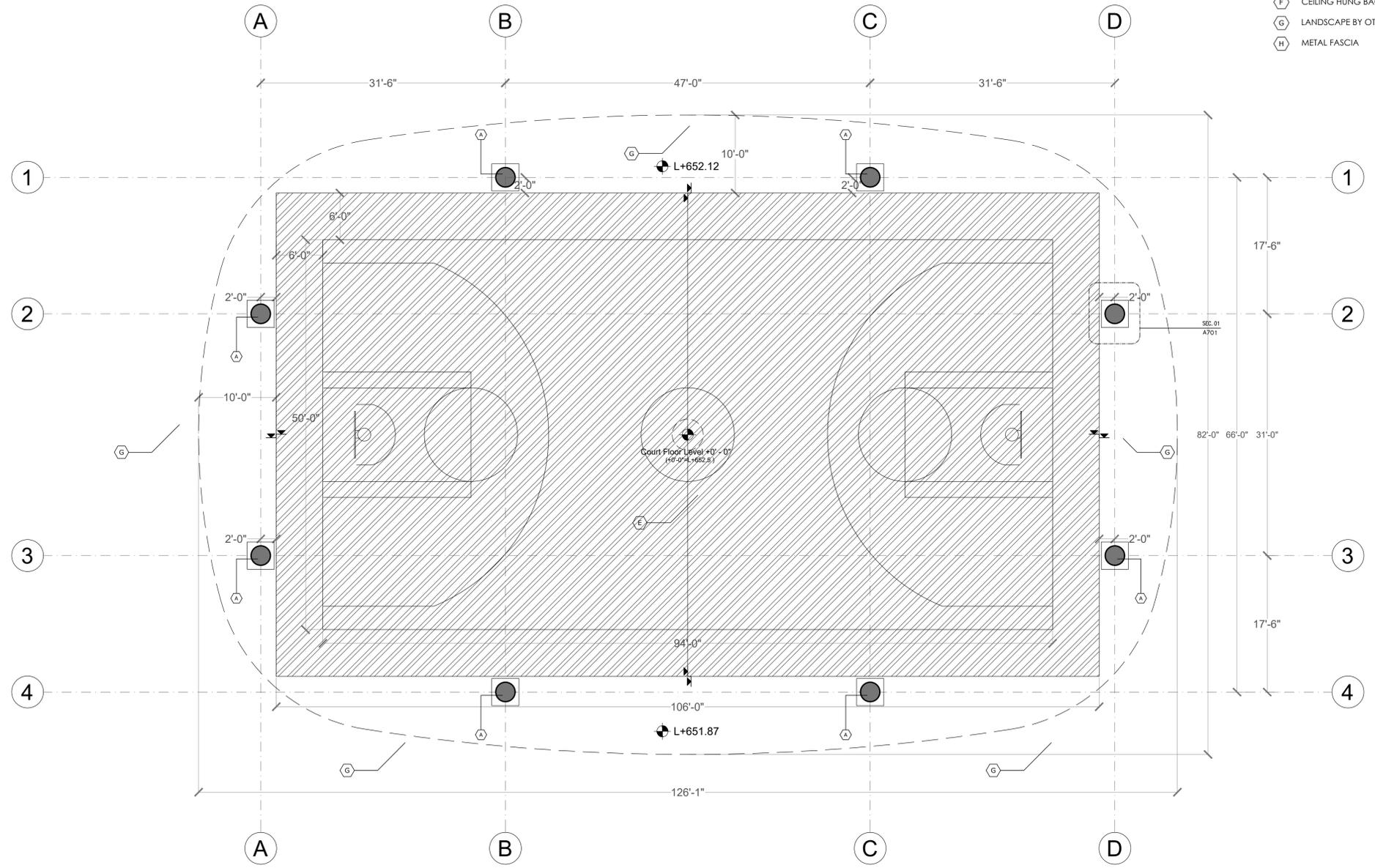
Date	Description

REVISIONS

No.	Date	Description
01	05/01/2023	BIDDING SET

KEY NOTES

- (A) METAL COLUMN-PROTECTIVE ENAMEL FLAT EXTERIOR PAINT
- (B) METAL PANEL FURROUT
- (C) METAL SOFFIT
- (D) METAL DECK + TPO
- (E) SPORTSMASTER COLORPLUS SYSTEM ON CONCRETE COLOR
- (F) CEILING HUNG BACKSTOP
- (G) LANDSCAPE BY OTHERS (NOT IN SCOPE)
- (H) METAL FASCIA



Court Area: 6572.00 Sq. Ft

1 Finish Floor Plan
SCALE: 1/8" = 1'-0"



NOT FOR CONSTRUCTION

Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR COORDINATION ONLY

SHEET NAME
FINISH FLOOR PLAN

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	05 / 01 / 2023	Code

SCALE
SHEET NUMBER
A120

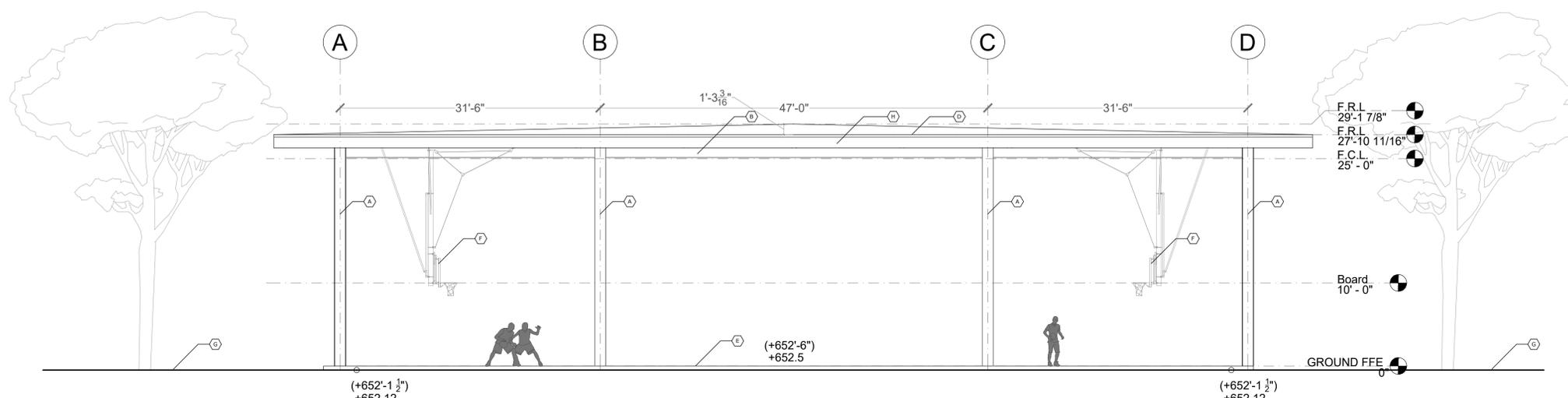
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Date	Description

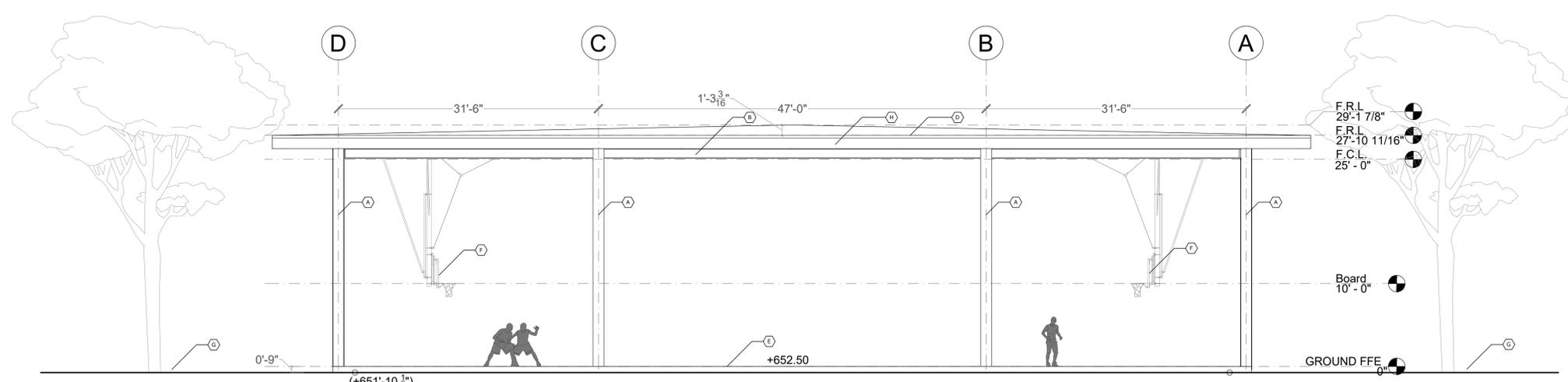
No.	Date	Description
01	05/01/2023	BIDDING SET

KEY NOTES

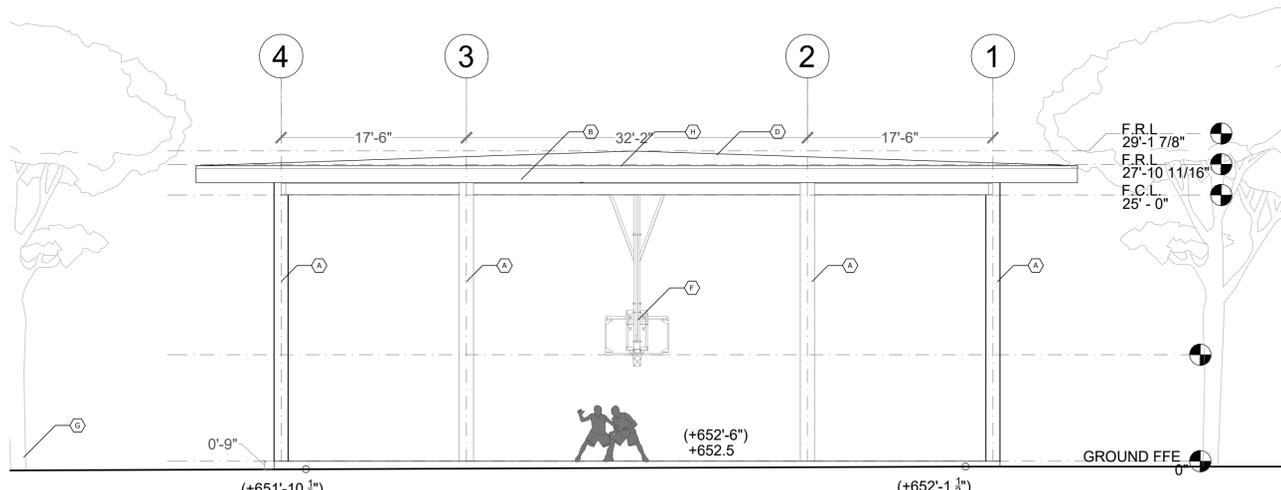
- (A) METAL COLUMN-PROTECTIVE ENAMEL FLAT EXTERIOR PAINT
- (B) METAL PANEL FURROUT
- (C) METAL SOFFIT
- (D) METAL DECK + TPO
- (E) SPORTSMaster COLORPLUS SYSTEM ON CONCRETE COLOR
- (F) CEILING HUNG BACKSTOP
- (G) LANDSCAPE BY OTHERS (NOT IN SCOPE)
- (H) METAL FASCIA



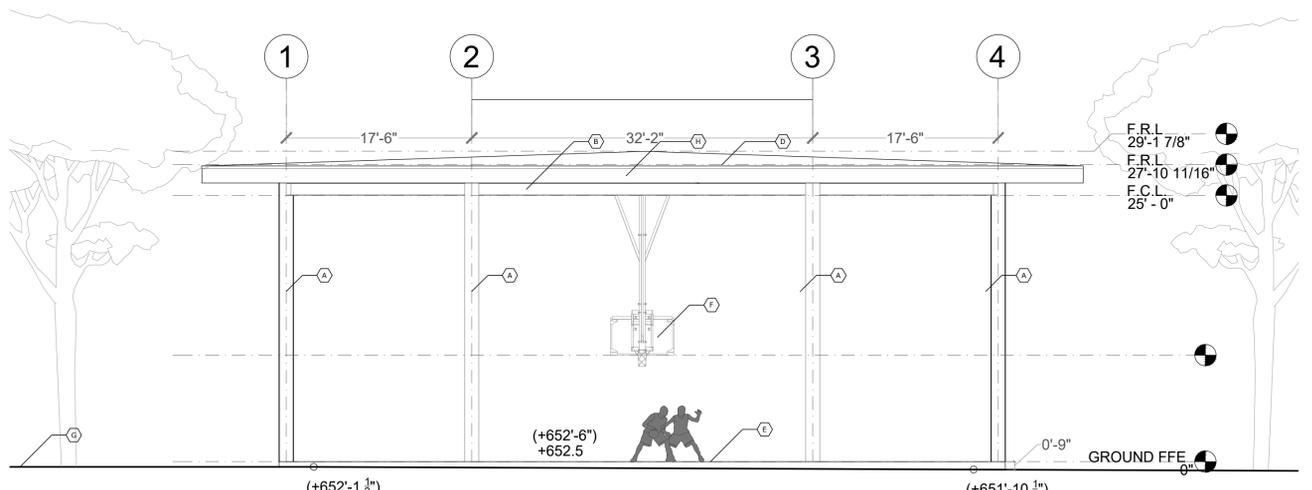
4 North Elevation
SCALE: 1/8" = 1'-0"



3 South Elevation
SCALE: 1/8" = 1'-0"



2 West Elevation
SCALE: 1/8" = 1'-0"



1 East Elevation
SCALE: 1/8" = 1'-0"

NOT FOR CONSTRUCTION

Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

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SHEET NAME
ELEVATIONS

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	05 / 01 / 2023	Code

SCALE
SHEET NUMBER

A201

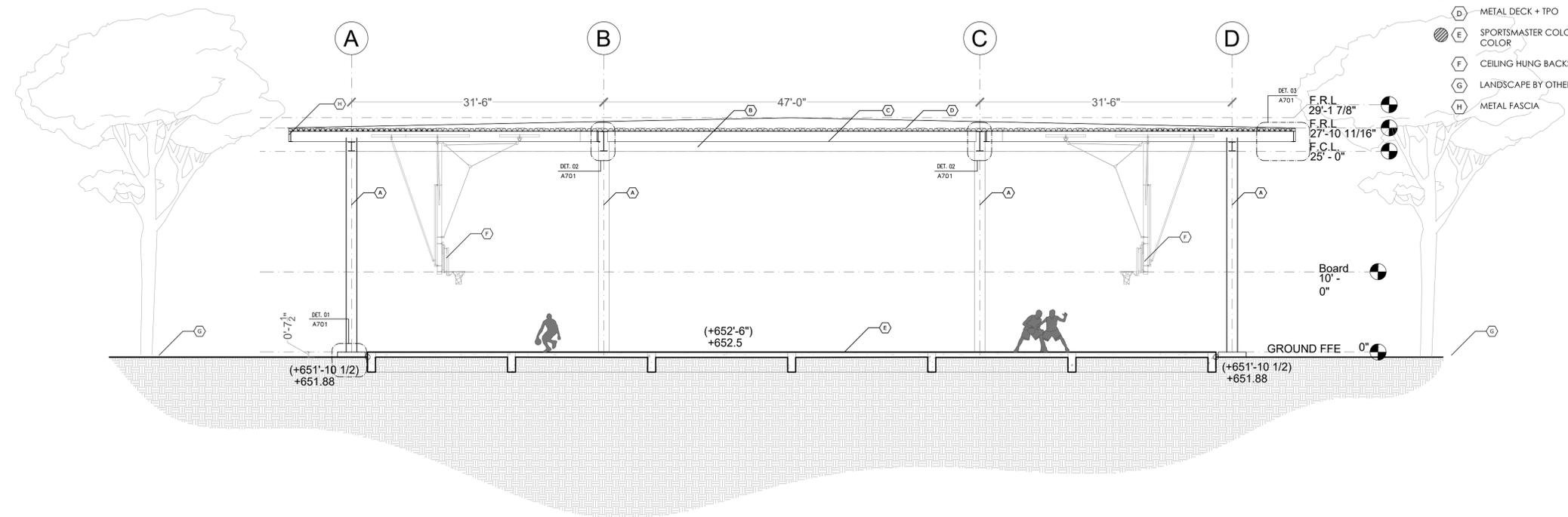
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Date	Description

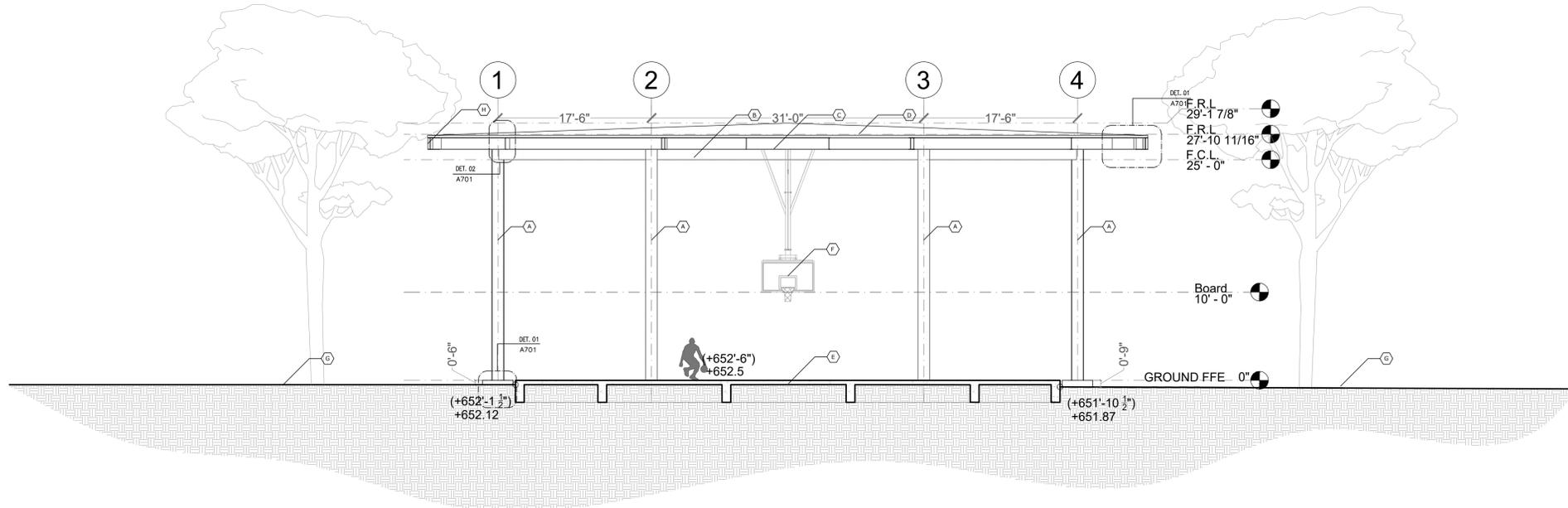
No.	Date	Description
01	05/01/2023	BIDDING SET

KEY NOTES

- (A) METAL COLUMN-PROTECTIVE ENAMEL FLAT EXTERIOR PAINT
- (B) METAL PANEL FURROUT
- (C) METAL SOFFIT
- (D) METAL DECK + TPO
- (E) SPORTSMASTER COLORPLUS SYSTEM ON CONCRETE COLOR
- (F) CEILING HUNG BACKSTOP
- (G) LANDSCAPE BY OTHERS (NOT IN SCOPE)
- (H) METAL FASCIA



2 Section
SCALE: 1/8" = 1'-0"



1 Section
SCALE: 1/8" = 1'-0"

Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

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SHEET NAME
SECTIONS

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	05 / 01 / 2023	Code

SCALE
SHEET NUMBER

A301

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Date	Description

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

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SHEET NAME

FINISH SCHEDULE

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	05 / 01 / 2023	Code

SCALE

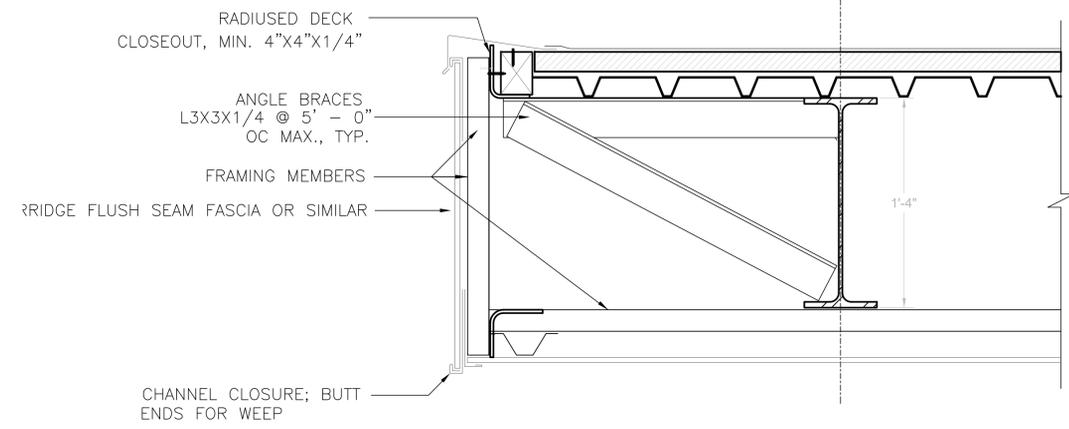
SHEET NUMBER

A610

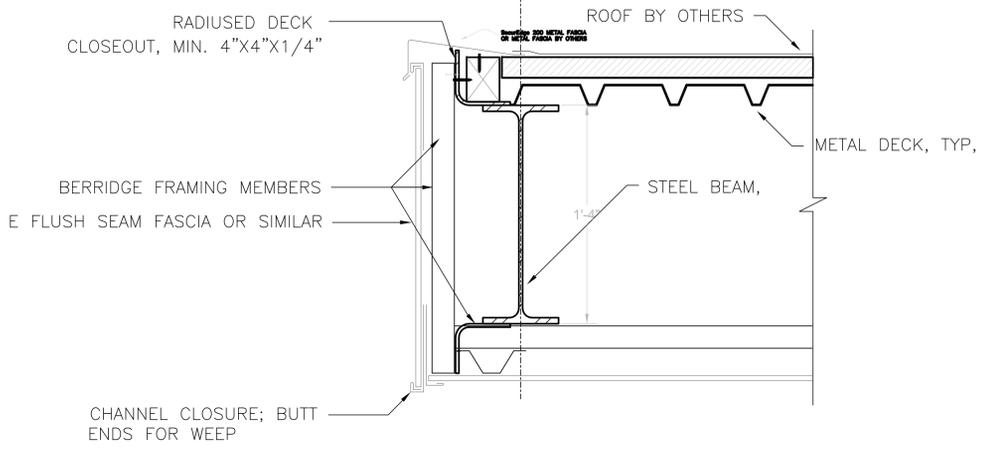
Preliminary Architectural finishes_rev00		Description	Brand	Contact	Website
Court surface color coating system					
1		Colorplus system 100% acrylic Neutral Concentrate (available with or without sand) and the ColorPlus Pigment Dispersion 1 coat of acrylic resurfacer, 2 coats of color	Sport master-sport surfaces	10692 I-10 Frontage Rd. Marion, TX 78124 (830) 420-3335	https://www.sportmaster.net/
Paint on steel structure					
1		High Performance Protective Enamel Flat Black Oil-Based Interior/Exterior Paint	Rust-Oleum Professional		https://www.rustoleum.com/product-catalog/consumer-brands/professional/high-performance-protective-enamel
2 (alternate)		BEHR PREMIUM® DIRECT-TO-METAL SEMI-GLOSS PAINT NO. 3200			
Cellings/Soffit					
1		Flush Seam metal Panel, stucco embossed 24 gauge steel 3 7/8" wide panel Concealed fasteners	Berridge	Berridge Manufacturing Company 6515 Fratt Rd San Antonio, TX. 78218 (210) 650-3050 Office www.berridge.com PBerridge@Berridge.com	https://www.berridge.com/products/fascia-wall-and-soffit-panels/flush-seam-panel/
2 (alternate)		Stretch ceiling PVC stretch film White / About 200 g/m2	Velum	VELUM DESIGN LLC www.velumdesign.com O. +1 833 33 83586 C. +1 786 617 7447 Main Office 17100 Collins Ave Sunny Isles Beach, FL 33160-Suite 210 mariana@velumdesign.com	www.velumdesign.com
3 (alternate)		Tensoshade HDP (High-Density Polyethylene)	Tensoshade	Tensoshade Irving Allande: irving@tensoshade.com	https://www.tensoshade.com/
Roof					
1		Carlisle's 16-Foot Sure-Weld, TPO Membrane, TPO Membrane	Carlisle	800-479-6832 P.O. Box 7000 Carlisle, PA 17013	www.carlisesyntec.com
a		Securshield Polyiso tapered insulation	Carlisle	800-479-6832 P.O. Box 7000 Carlisle, PA 17013	www.carlisesyntec.com
b					
Fascia/trim					
		Not included. It depends on soffit system. GC to provide			
Framing for soffit					
		Not included in some options, GC to ensure pricing			
Basketball backstops/backboard/rims					
1		Single Post Rear-Braced Front-Folding Ceiling Hung Basketball Backstop		https://www.garedsports.com/products/single-post-rear-braced-front-folding-ceiling-hung-basketball-backstop-0	Jared Jellison President Office: 512-282-5256 jared@jelcogyms.com
2 (alternate)		Spalding Gliding Rear-Braced Ceiling Mast Basketball Backstop	Sports unlimited	800.693.6368	https://www.sportsunlimitedinc.com/spalding-gliding-rear-braced-ceiling-mast-basketball-backstop.html
		Spalding SuperGlass Collegiate Basketball Backboard, Rim, & Padding Package		www.sportsunlimitedinc.com	https://www.sportsunlimitedinc.com/bpipackage3.html
3 (alternate)		Folding Ceiling Hung Basketball Backstop Professional PRO-MOLD® Backboard Padding Collegiate Premium Breakaway Basketball Rim	Paddock Bleacher Services	1475 CANAL STREET P.O. Box 175 LOCKPORT IL, 60441 https://www.paddockbleachers.com/basketball-backstops/	www.paddockbleachers.com
Water fountain					
1		Elkay Freestanding Tubular ADA Drinking Fountain w/Pet Bowl Model Number: LK4420DBBLK	Elkay		https://www.restroomdirect.com/elkay-LK4420DB-drinking-fountain.aspx

NOT FOR CONSTRUCTION

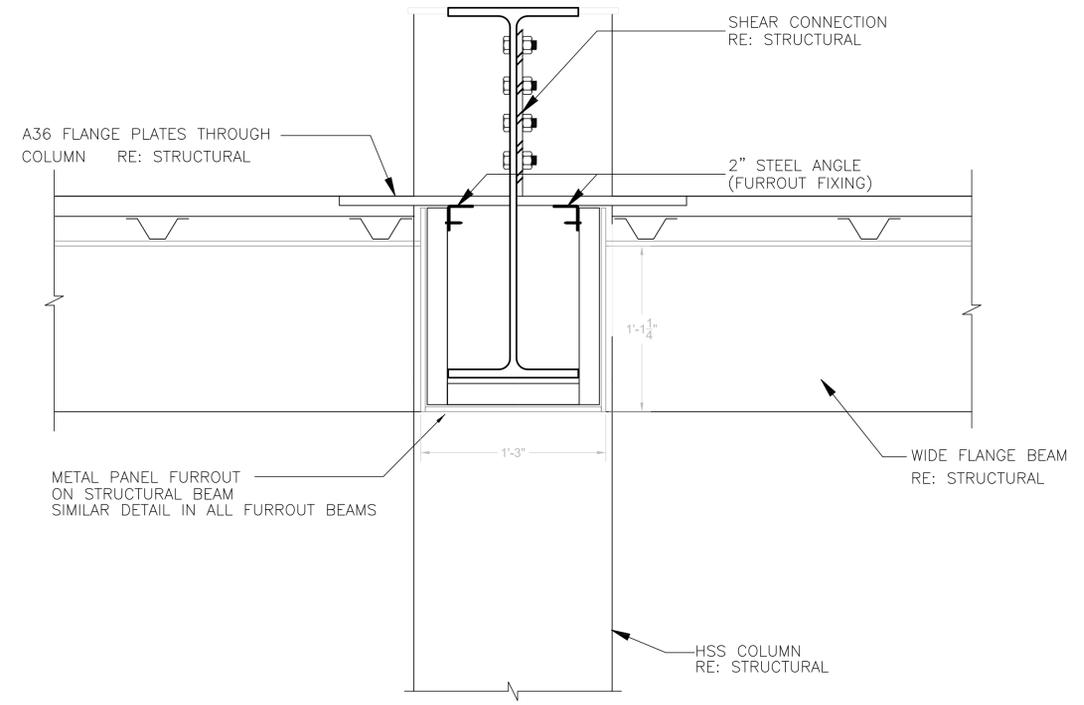
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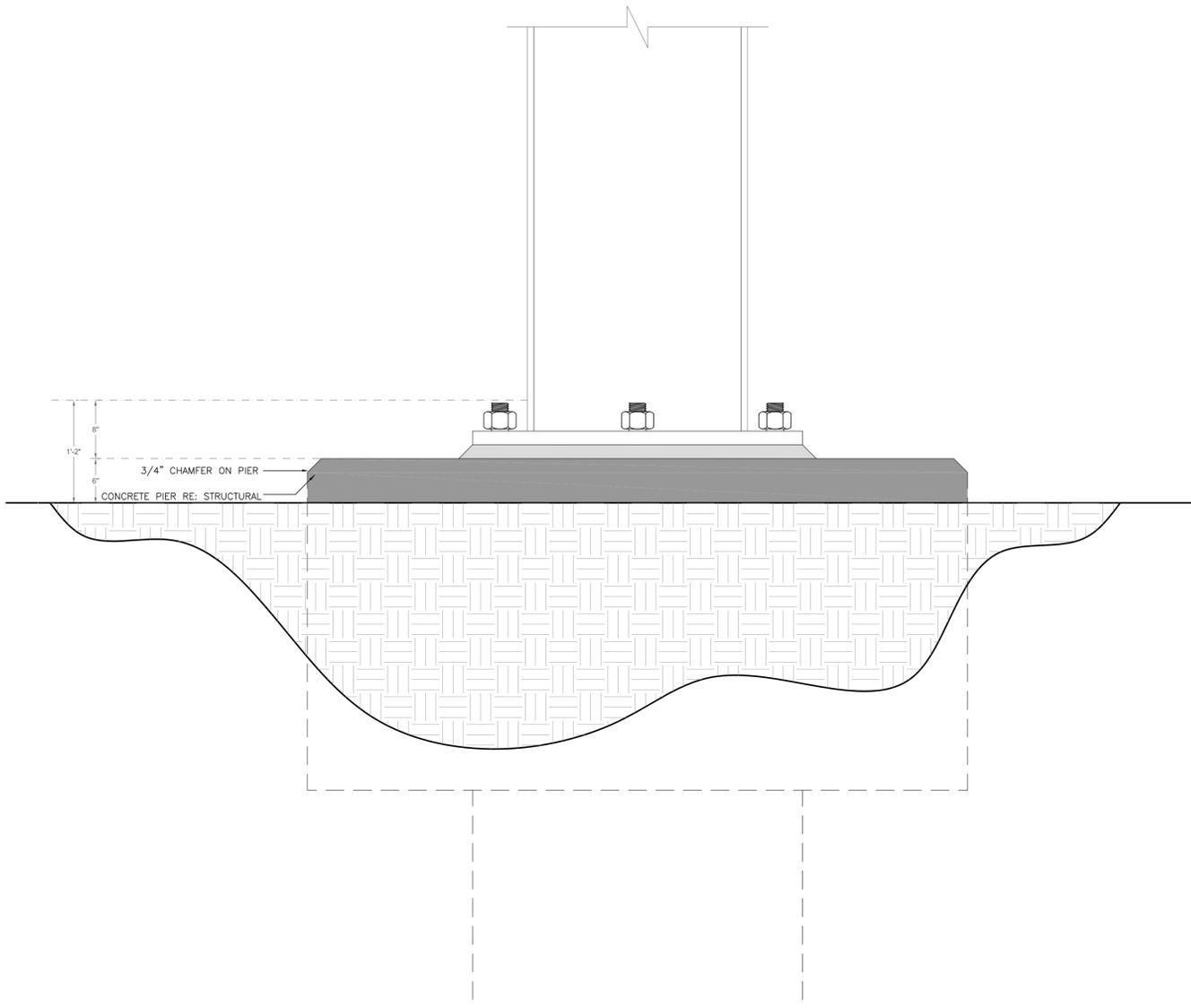
4 Roof Deck Long Overhang
SCALE: N.T.S



3 Closeout At Roof
SCALE: N.T.S



2 Structural Beams Furrou
SCALE: N.T.S



1 Straight Shaft Pier & Pier Cap
SCALE: N.T.S

NOT FOR CONSTRUCTION

ISSUED SETS

Date	Description

REVISIONS

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion
Hemisfair
San Antonio, TX. 78205

PROFESSIONAL SEAL

FOR COORDINATION ONLY

SHEET NAME
DETAILS

Draftsman	Phase	DESIGN DEVELOPMENT
GVI Approval	Project No.	22118
Client Approval	File	
Date	05 / 01 / 2023	Code

SCALE
SHEET NUMBER

A701

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1	2	3	4	5	6	7	8	9	10	11	12	13	14	15		
L	ABBREVIATION LIST		DESIGN CRITERIA		STRUCTURAL SUBMITTALS		SPECIAL INSPECTIONS AND REPORTS		GENERAL FOUNDATION							
	ACP	AUGER CAST PILE	MIN.	MINIMUM	1.	THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE (IBC), 2021 EDITION.	1.	SUBMIT TO THE ENGINEER FOR REVIEW APPROPRIATE SCHEDULES, SHOP DRAWINGS, SAMPLES, TEST REPORTS, AND PRODUCT DATA THAT IS RELATED TO THE STRUCTURAL PORTION OF THE WORK ACCORDING TO AIA DOCUMENT A201 GENERAL CONDITIONS OF THE CONTRACT FOR CONSTRUCTION. NO WORK SHALL BE FABRICATED UNTIL THE ENGINEER'S REVIEW HAS BEEN OBTAINED. PROVIDED IS A LIST OF STRUCTURAL SUBMITTALS REQUIRED FOR THIS PROJECT, AND REFER TO SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS:	1.	SPECIAL INSPECTIONS AND TESTING SHALL BE DONE IN ACCORDANCE WITH THE STATEMENT OF SPECIAL INSPECTIONS AND TESTING TO BE APPLIED PER THE FOLLOWING CRITERIA.	1.	DIMENSIONS OF FOUNDATION ELEMENTS INDICATE MINIMUM ACCEPTABLE SIZES. LARGER SIZES FORMED BY LESS ACCURATE CONSTRUCTION MAY REQUIRE ADDITIONAL REINFORCING NOT SHOWN, WHICH SHALL BE DETERMINED BY THE STRUCTURAL ENGINEER DURING THE CONSTRUCTION OBSERVATION PROCESS. CUT HAUNCHES ON EACH SIDE OF TRENCHES OF ADEQUATE SIZE TO MAINTAIN THE VERTICAL SIDES OF THE TRENCH.				
	ADD.	ADDITIONAL	MISC.	MISCELLANEOUS	2.	DEAD LOADS:	A.	DESIGN DEAD LOADS INCLUDE THE WEIGHT OF THE STRUCTURE, MATERIALS, COMPONENTS, PERMANENT FIXTURES, AND 4 PSF MECHANICAL DUCT ALLOWANCE.	2.	THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE (RDPRIC) FOR THIS PROJECT SHALL BE DESIGNATED BY THE OWNER. SUBMIT ALL SPECIAL INSPECTION REPORTS DIRECTLY TO THE RDPRIC AND BUILDING OFFICIAL FOR REVIEW. THE RDPRIC SHALL FORWARD ALL THE STRUCTURALLY RELATED SPECIAL INSPECTION REPORTS TO THE STRUCTURAL ENGINEER FOR REVIEW.	2.	GRADE BEAMS AND FOOTINGS SHALL BEAR A MINIMUM OF 12" INTO COMPACTED STRUCTURAL FILL OR COMPETENT NATIVE SOILS. REDUCED PENETRATION DEPTHS INTO BEDROCK SHALL BE PER THE GEOTECHNICAL REPORT OR A MINIMUM OF 3". WHERE NOTED, FOUNDATIONS SHALL BE FULLY SET ON APPROVED VOID FORMS.				
	ALT.	ALTERNATIVE	N/A	NOT APPLICABLE	B.	LOADING FOR MECHANICAL AND ELECTRICAL EQUIPMENT IS BASED ON THE WEIGHTS OF ASSUMED EQUIPMENT AS INDICATED ON THE STRUCTURAL DRAWINGS (INCLUDING THE WEIGHT OF CONCRETE PADS WHERE INDICATED ON MEP DRAWINGS). ANY DISCREPANCIES OR CHANGES IN THE TYPE, SIZE, LOCATION, OR NUMBER OF PIECES OF EQUIPMENT SHOULD BE REPORTED TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO PLACEMENT OF EQUIPMENT.	B.	FABRICATION / ERECTION DRAWINGS:	3.	SPECIAL INSPECTORS SHALL BE CONTRACTED BY THE OWNER OR THE OWNER'S AUTHORIZED AGENT. SPECIAL INSPECTORS SHALL BE QUALIFIED PER THE REQUIREMENTS LISTED IN SECTION 1704.2.	3.	PLACE MEP LINES BELOW SLABS AND OUTSIDE OF GRADE BEAMS AND FOOTINGS. DO NOT PLACE LINES PARALLEL WITHIN OR PARALLEL BELOW GRADE BEAMS AND FOOTINGS. REFERENCE TYPICAL DETAILS FOR ALLOWABLE PENETRATIONS PERPENDICULAR TO GRADE BEAMS, FOOTINGS AND SLABS. PROVIDE PROTECTION OF MEP LINES CROSSING GRADE BEAMS OR PROJECTING THROUGH THE SLAB TO ALLOW FOR FOUNDATION MOVEMENT.				
	ARCH.	ARCHITECT, ARCHITECTURAL	NTS	NOT TO SCALE	C.	LIVE LOADS:	C.	PRODUCT DATA SUBMITTALS	4.	THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TESTING, INSPECTIONS, AND NOTIFYING THE ARCHITECT/ENGINEER, SPECIAL INSPECTORS, AND BUILDING OFFICIAL PER SECTION 110.3, OF WORK READY FOR INSPECTION. THE GENERAL CONTRACTOR MUST PROVIDE ACCESS TO AND MEANS FOR PROPER INSPECTION OF SUCH WORK.	4.	AT A MINIMUM THE VAPOR RETARDER SHALL CONFORM TO IBC "CLASS I" WITH A PERMEANCE OF 0.1 PERMS OR LESS, ASTM E1745 "CLASS C", AND ACI 302.2R WITH A MINIMUM THICKNESS OF 10 MIL WHERE ARCHITECTURAL PLANS CALL FOR SENSITIVE FLOOR MATERIALS, A VAPOR RETARDER EXCEEDING THE ABOVE SPECIFICATIONS MAY BE REQUIRED.				
	ASI	ARCHITECT'S SUPPLEMENTAL INSTRUCTIONS	NS	NON-SHRINK	D.	FOR LIVE LOADS EXCEEDING 100 PSF, NO REDUCTION HAS BEEN MADE, EXCEPT THAT THE DESIGN LIVE LOADS ARE BASED ON THE MORE RESTRICTIVE OF THE UNIFORM LOAD LISTED BELOW OR THE CONCENTRATED LOAD LISTED ACTING OVER AN AREA 2' - 6" SQUARE OR, IN THE CASE OF PARKING GARAGES 20 IN. OR STAIR TREADS, 4 IN.	D.	REPORTS:	5.	THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED, MOST CURRENT DESIGN DOCUMENTS AND SPECIFICATIONS, AND SHALL PROVIDE REPORTS TO THE BUILDING OFFICIAL, THE ARCHITECT/ENGINEER, AND OTHER DESIGNATED PERSONS.	5.	EXTEND FORMWORK AT LEAST 6" BELOW THE FINISHED GRADE ELEVATION AT PERIMETER BEAMS.				
	BOS	BOTTOM OF STEEL	NTE	NOT TO EXCEED	4.	RISK CATEGORY OF BUILDING:	D.	REPORTS:	6.	THE SPECIAL INSPECTOR SHALL REPORT ALL DISCREPANCIES TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN IF UNCORRECTED, TO THE PROPER DESIGN AUTHORITY AND TO THE BUILDING OFFICIAL.	6.	VAPOR RETARDERS SHALL BE INSTALLED IN ACCORDANCE WITH ASTM E1643, WITH THE MATERIAL CONTINUOUS BELOW FOUNDATION CONCRETE AREAS AND WITH JOINTS LAPPED AT LEAST 6", OR AS INSTRUCTED BY THE MANUFACTURER.				
	BP	BASE PLATE	NWC	NORMAL WEIGHT CONCRETE	5.	WIND DESIGN CRITERIA:	D.	REPORTS:	7.	THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED, MOST CURRENT DESIGN DOCUMENTS AND SPECIFICATIONS, AND SHALL PROVIDE REPORTS TO THE BUILDING OFFICIAL, THE ARCHITECT/ENGINEER, AND OTHER DESIGNATED PERSONS.	7.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
	CJ	CONTROL JOINT	OC	ON CENTER EACH WAY	6.	ULTIMATE DESIGN WIND SPEED (3 SEC. GUST, V _{ult})	D.	REPORTS:	8.	ADDITIONAL INSPECTIONS MAY BE REQUIRED BY THE GOVERNING JURISDICTION. THE BELOW REQUIREMENTS ARE MINIMUM PROJECT STANDARDS.	8.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
	CIP	COMPLETE JOINT PENETRATION	OP	OPPOSITE	7.	NOMINAL DESIGN WIND SPEED (V ₅₀)	D.	REPORTS:	9.	STRUCTURAL OBSERVATIONS SHALL BE PERFORMED BY A LICENSED DESIGN PROFESSIONAL OR THEIR REPRESENTATIVE DURING PERIODIC SITE VISITS FOR THE WORK LISTED BELOW. THIS DOES NOT CONSTITUTE A SPECIAL INSPECTION.	9.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
	CL	CENTER LINE	PI	PLASTICITY INDEX	8.	EXPOSURE CATEGORY	D.	REPORTS:	10.	CONCRETE CONSTRUCTION	10.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
	CMU	CORNER CONCRETE MASONRY UNIT	PLF	LBS/ LINEAR FOOT	9.	INTERNAL PRESSURE COEFFICIENT (GC _{pi})	D.	REPORTS:	11.	SOILS INSPECTION	11.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
	COL	COLUMN	PSF	LBS/ SQUARE FOOT	10.	DIRECTIONALITY FACTOR (K _d)	D.	REPORTS:	A.	SPECIAL INSPECTIONS PER IBC SECTION 1705.3	12.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
	COMP.	COMPOSITE	PSI	LBS/ SQUARE INCH	11.	TOPOGRAPHY FACTOR (K _e)	D.	REPORTS:	B.	STRUCTURAL OBSERVATIONS:	13.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
	CONT.	CONTINUOUS	REC.	RECOMMENDED	12.	BASIC WIND PRESSURE, (q _n)	D.	REPORTS:	1.	OBSERVE PLACEMENT OF REINFORCING STEEL, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS PRIOR TO PLACEMENT OF CONCRETE.	14.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.				
CU	CUBIC	REF.	REFERENCE	13.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	2.	WELDING OF STEEL REINFORCEMENT.	15.	EXPANSION JOINTS SHALL BE FORMED BY A BITUMINOUS FILLER MATERIAL, COMPLYING WITH ASTM D1751, ASPHALT-SATURATED CELLULOSE FIBER. SET 1/2" - 1" BELOW THE SURFACE IN ORDER TO FILL THE JOINT WITH A FLEXIBLE JOINT FILLER. EXTERIOR JOINTS SHALL BE SEALED WITH A TRAFFIC GRADE SEALANT.					
Ø	DIAMETER	REQ.	REQUIRED	14.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	3.	PLACEMENT OF CONCRETE.							
DF	DOUGLAS FIR	REQ.	REQUIRED	15.	DESIGN BASE SHEAR, (V)	D.	REPORTS:	4.	FABRICATION OF SPECIMENS FOR STRENGTH TEST; MINIMUM (1) SET FOR 100 YDS.							
EA	EACH	REQ.	REQUIRED	16.	SEISMIC RESPONSE COEFFICIENT (C _s)	D.	REPORTS:	5.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
EJ	EXPANSION JOINT	REQ.	REQUIRED	17.	ANALYSIS PROCEDURE USED	D.	REPORTS:	6.	CONTINUOUS INSPECTIONS:							
EMBED.	EMBEDMENT	REQ.	REQUIRED	18.	SNOW DESIGN CRITERIA:	D.	REPORTS:	a.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
EN	EDGE NAILING	REQ.	REQUIRED	19.	GROUND SNOW LOAD (P _g)	D.	REPORTS:	b.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
EOR	ENGINEER OF RECORD	REQ.	REQUIRED	20.	FLAT-ROOF SNOW LOAD (P _f)	D.	REPORTS:	c.	INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF COMPACTED FILL.							
EP	EMBED PLATE	REQ.	REQUIRED	21.	EXPOSURE FACTOR (C _e)	D.	REPORTS:	d.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
EQ	EQUAL	REQ.	REQUIRED	22.	IMPORTANCE FACTOR (I _s)	D.	REPORTS:	e.	VERIFY CURING PROCEDURES AND MAINTENANCE OF CURING TEMPERATURE.							
EQUIP.	EQUIPMENT	REQ.	REQUIRED	23.	THERMAL FACTOR (C _t)	D.	REPORTS:	f.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
EXT.	EXTERIOR	REQ.	REQUIRED	24.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	1.	OBSERVE PLACEMENT OF REINFORCING STEEL, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS PRIOR TO PLACEMENT OF CONCRETE.							
FF	FINISH FLOOR ELEVATION	REQ.	REQUIRED	25.	BASIC SEISMIC-FORCE-RESISTING SYSTEM	D.	REPORTS:	2.	WELDING OF STEEL REINFORCEMENT.							
GA	GAUGE	REQ.	REQUIRED	26.	DESIGN BASE SHEAR, (V)	D.	REPORTS:	3.	PLACEMENT OF CONCRETE.							
GALV.	GALVANIZED	REQ.	REQUIRED	27.	SEISMIC RESPONSE COEFFICIENT (C _s)	D.	REPORTS:	4.	FABRICATION OF SPECIMENS FOR STRENGTH TEST; MINIMUM (1) SET FOR 100 YDS.							
GC	GENERAL CONTRACTOR	REQ.	REQUIRED	28.	ANALYSIS PROCEDURE USED	D.	REPORTS:	5.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
GFCMU	GROUT FILLED CONCRETE MASONRY UNIT	REQ.	REQUIRED	29.	SNOW DESIGN CRITERIA:	D.	REPORTS:	6.	CONTINUOUS INSPECTIONS:							
GR.	GRADE	REQ.	REQUIRED	30.	GROUND SNOW LOAD (P _g)	D.	REPORTS:	a.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
HCA	HEADED CONCRETE ANCHOR	REQ.	REQUIRED	31.	FLAT-ROOF SNOW LOAD (P _f)	D.	REPORTS:	b.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
IN	INCHES	REQ.	REQUIRED	32.	EXPOSURE FACTOR (C _e)	D.	REPORTS:	c.	INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF COMPACTED FILL.							
INFO.	INFORMATION	REQ.	REQUIRED	33.	IMPORTANCE FACTOR (I _s)	D.	REPORTS:	d.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
INT.	INTERMEDIATE	REQ.	REQUIRED	34.	THERMAL FACTOR (C _t)	D.	REPORTS:	e.	VERIFY CURING PROCEDURES AND MAINTENANCE OF CURING TEMPERATURE.							
K	KIP (1,000 LBS)	REQ.	REQUIRED	35.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	1.	OBSERVE PLACEMENT OF REINFORCING STEEL, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS PRIOR TO PLACEMENT OF CONCRETE.							
KD	KILN DRIED	REQ.	REQUIRED	36.	BASIC SEISMIC-FORCE-RESISTING SYSTEM	D.	REPORTS:	2.	WELDING OF STEEL REINFORCEMENT.							
LBS	POUNDS	REQ.	REQUIRED	37.	DESIGN BASE SHEAR, (V)	D.	REPORTS:	3.	PLACEMENT OF CONCRETE.							
LG	LIGHT GAUGE	REQ.	REQUIRED	38.	SEISMIC RESPONSE COEFFICIENT (C _s)	D.	REPORTS:	4.	FABRICATION OF SPECIMENS FOR STRENGTH TEST; MINIMUM (1) SET FOR 100 YDS.							
LW	LIGHT WEIGHT	REQ.	REQUIRED	39.	ANALYSIS PROCEDURE USED	D.	REPORTS:	5.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
LSL	LAMINATED STRAND LUMBER	REQ.	REQUIRED	40.	SNOW DESIGN CRITERIA:	D.	REPORTS:	6.	CONTINUOUS INSPECTIONS:							
LVL	LAMINATED VENEER LUMBER	REQ.	REQUIRED	41.	GROUND SNOW LOAD (P _g)	D.	REPORTS:	a.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
LW	LIGHT WEIGHT	REQ.	REQUIRED	42.	FLAT-ROOF SNOW LOAD (P _f)	D.	REPORTS:	b.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
MAX.	MAXIMUM	REQ.	REQUIRED	43.	EXPOSURE FACTOR (C _e)	D.	REPORTS:	c.	INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF COMPACTED FILL.							
MECH.	MECHANICAL	REQ.	REQUIRED	44.	IMPORTANCE FACTOR (I _s)	D.	REPORTS:	d.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
MEP	MECHANICAL ELECTRICAL PLUMBING	REQ.	REQUIRED	45.	THERMAL FACTOR (C _t)	D.	REPORTS:	e.	VERIFY CURING PROCEDURES AND MAINTENANCE OF CURING TEMPERATURE.							
MFR	MANUFACTURER	REQ.	REQUIRED	46.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	1.	OBSERVE PLACEMENT OF REINFORCING STEEL, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS PRIOR TO PLACEMENT OF CONCRETE.							
MIL	0.001"	REQ.	REQUIRED	47.	BASIC SEISMIC-FORCE-RESISTING SYSTEM	D.	REPORTS:	2.	WELDING OF STEEL REINFORCEMENT.							
		REQ.	REQUIRED	48.	DESIGN BASE SHEAR, (V)	D.	REPORTS:	3.	PLACEMENT OF CONCRETE.							
		REQ.	REQUIRED	49.	SEISMIC RESPONSE COEFFICIENT (C _s)	D.	REPORTS:	4.	FABRICATION OF SPECIMENS FOR STRENGTH TEST; MINIMUM (1) SET FOR 100 YDS.							
		REQ.	REQUIRED	50.	ANALYSIS PROCEDURE USED	D.	REPORTS:	5.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
		REQ.	REQUIRED	51.	SNOW DESIGN CRITERIA:	D.	REPORTS:	6.	CONTINUOUS INSPECTIONS:							
		REQ.	REQUIRED	52.	GROUND SNOW LOAD (P _g)	D.	REPORTS:	a.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
		REQ.	REQUIRED	53.	FLAT-ROOF SNOW LOAD (P _f)	D.	REPORTS:	b.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
		REQ.	REQUIRED	54.	EXPOSURE FACTOR (C _e)	D.	REPORTS:	c.	INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF COMPACTED FILL.							
		REQ.	REQUIRED	55.	IMPORTANCE FACTOR (I _s)	D.	REPORTS:	d.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
		REQ.	REQUIRED	56.	THERMAL FACTOR (C _t)	D.	REPORTS:	e.	VERIFY CURING PROCEDURES AND MAINTENANCE OF CURING TEMPERATURE.							
		REQ.	REQUIRED	57.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	1.	OBSERVE PLACEMENT OF REINFORCING STEEL, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS PRIOR TO PLACEMENT OF CONCRETE.							
		REQ.	REQUIRED	58.	BASIC SEISMIC-FORCE-RESISTING SYSTEM	D.	REPORTS:	2.	WELDING OF STEEL REINFORCEMENT.							
		REQ.	REQUIRED	59.	DESIGN BASE SHEAR, (V)	D.	REPORTS:	3.	PLACEMENT OF CONCRETE.							
		REQ.	REQUIRED	60.	SEISMIC RESPONSE COEFFICIENT (C _s)	D.	REPORTS:	4.	FABRICATION OF SPECIMENS FOR STRENGTH TEST; MINIMUM (1) SET FOR 100 YDS.							
		REQ.	REQUIRED	61.	ANALYSIS PROCEDURE USED	D.	REPORTS:	5.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
		REQ.	REQUIRED	62.	SNOW DESIGN CRITERIA:	D.	REPORTS:	6.	CONTINUOUS INSPECTIONS:							
		REQ.	REQUIRED	63.	GROUND SNOW LOAD (P _g)	D.	REPORTS:	a.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
		REQ.	REQUIRED	64.	FLAT-ROOF SNOW LOAD (P _f)	D.	REPORTS:	b.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
		REQ.	REQUIRED	65.	EXPOSURE FACTOR (C _e)	D.	REPORTS:	c.	INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF COMPACTED FILL.							
		REQ.	REQUIRED	66.	IMPORTANCE FACTOR (I _s)	D.	REPORTS:	d.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
		REQ.	REQUIRED	67.	THERMAL FACTOR (C _t)	D.	REPORTS:	e.	VERIFY CURING PROCEDURES AND MAINTENANCE OF CURING TEMPERATURE.							
		REQ.	REQUIRED	68.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	1.	OBSERVE PLACEMENT OF REINFORCING STEEL, ANCHOR RODS, AND OTHER EMBEDDED COMPONENTS PRIOR TO PLACEMENT OF CONCRETE.							
		REQ.	REQUIRED	69.	BASIC SEISMIC-FORCE-RESISTING SYSTEM	D.	REPORTS:	2.	WELDING OF STEEL REINFORCEMENT.							
		REQ.	REQUIRED	70.	DESIGN BASE SHEAR, (V)	D.	REPORTS:	3.	PLACEMENT OF CONCRETE.							
		REQ.	REQUIRED	71.	SEISMIC RESPONSE COEFFICIENT (C _s)	D.	REPORTS:	4.	FABRICATION OF SPECIMENS FOR STRENGTH TEST; MINIMUM (1) SET FOR 100 YDS.							
		REQ.	REQUIRED	72.	ANALYSIS PROCEDURE USED	D.	REPORTS:	5.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
		REQ.	REQUIRED	73.	SNOW DESIGN CRITERIA:	D.	REPORTS:	6.	CONTINUOUS INSPECTIONS:							
		REQ.	REQUIRED	74.	GROUND SNOW LOAD (P _g)	D.	REPORTS:	a.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
		REQ.	REQUIRED	75.	FLAT-ROOF SNOW LOAD (P _f)	D.	REPORTS:	b.	VERIFY CONCRETE STRENGTH BEFORE REMOVAL OF CURES AND FORMS FROM BEAMS AND SLABS.							
		REQ.	REQUIRED	76.	EXPOSURE FACTOR (C _e)	D.	REPORTS:	c.	INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY PRIOR TO PLACEMENT OF COMPACTED FILL.							
		REQ.	REQUIRED	77.	IMPORTANCE FACTOR (I _s)	D.	REPORTS:	d.	PERFORM CLASSIFICATION AND TESTING OF MATERIALS TO BE USED FOR FILL.							
		REQ.	REQUIRED	78.	THERMAL FACTOR (C _t)	D.	REPORTS:	e.	VERIFY CURING PROCEDURES AND MAINTENANCE OF CURING TEMPERATURE.							
		REQ.	REQUIRED	79.	SEISMIC DESIGN CATEGORY	D.	REPORTS:	1.	OBSERVE PLACEMENT OF REINFORCING STEEL, ANCHOR RODS, AND OTHER EMBEDDED COMPONENT							

CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO THE FOLLOWING STANDARDS AND ANY STANDARDS REFERENCED THEREIN:
 - ACI 301 - SPECIFICATIONS FOR STRUCTURAL CONCRETE
 - ACI 117 - SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS
 - ACI 318 - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
- CONCRETE SHALL BE IN ACCORDANCE WITH THE TABLES BELOW UNLESS NOTED OTHERWISE.

CONCRETE MIX SCHEDULE					
CLASS	28 DAY STRENGTH (f' _c) (PSI)	MAX. W/C	SLUMP	MAX. AGGREGATE SIZE	AIR CONTENT
A	3,000	0.55	5" - 7"	1 1/2"	3% - 6%
B	3,000	0.55	4" - 6"	1 1/2"	3% - 6%
C	4,000	0.55	4" - 6"	1 1/2"	3% - 6%
D	4,000	0.45	4" - 6"	1 1/2"	≤ 1.5%
E	5,000	0.40	3" - 5"	1"	6% - 10%
F	4,000	0.50	4" - 6"	3/4"	-
G	3,000	0.50	4" - 6"	3/4"	-
H	2,000	0.55	5" - 7"	3/4"	-

- NOTES:**
- CONCRETE SHALL BE NORMAL WEIGHT UNLESS NOTED OTHERWISE.
 - FLY ASH MAY BE USED UP TO 25% REPLACEMENT OF PORTLAND CEMENT, EXCEPT AT POLISHED SLABS (LIMITED TO 15%) OR ARCHITECTURALLY EXPOSED CONCRETE (VERIFY WITH ARCHITECT).
 - ALL MIXES SHALL UTILIZE A WATER REDUCING ADMIXTURE.
 - CURING COMPOUNDS ARE NOT ACCEPTABLE FOR POLISHED SLAB APPLICATIONS. SLUMP SHALL BE DETERMINED AT POINT OF PLACEMENT.
 - FOR TOPPING SLABS LESS THAN 2" THICK, CONTRACTOR SHALL SUBMIT PROPRIETARY MIX DESIGN AND PREPARATION PROCEDURE FOR APPROVAL.

USE	CLASS
DRILLED PIERS	A
PIER CAPS	B
GRADE BEAMS	A
SLABS-ON-GRADE	B

- MATERIALS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:
 - READY-MIXED CONCRETE - ASTM C94
 - PORTLAND CEMENT - ASTM C150, TYPE III
 - FLY ASH - ASTM C618, CLASS F OR C
 - NORMAL WEIGHT AGGREGATES - ASTM C33
 - LIGHT WEIGHT AGGREGATES - ASTM C330
 - WATER - ASTM C1602
 - WATER-REDUCING, PLASTICIZING, AND RETARDING ADMIXTURE - ASTM C494
 - AIR ENTRAINING ADMIXTURE - ASTM C260
 - CURING COMPOUNDS - ASTM C309, TYPE 1, CLASS B
 - FLOOR SEALERS, HARDENERS, FINISHES, AND COVERINGS SHALL BE COMPATIBLE WITH CONCRETE PROPERTIES

- READY-MIXED CONCRETE SHALL BE FURNISHED WITH BATCH TICKET INFORMATION. PROJECT-SITE MIXING IS NOT ACCEPTABLE.
- PLACEMENT OF CONCRETE SHALL BE COMPLETED WITHIN 90 MINUTES AFTER THE INTRODUCTION OF THE MIXING WATER, PER ASTM C94.

- COLD WEATHER CONCRETE PLACEMENT SHALL COMPLY WITH ACI 306.1 AND AS FOLLOWS:
 - WHEN AVERAGE HIGH AND LOW TEMPERATURE IS EXPECTED TO FALL BELOW 40° F FOR (3) CONSECUTIVE DAYS, MAINTAIN DELIVERED CONCRETE MIX TEMPERATURE WITHIN THE TEMPERATURE RANGE REQUIRED BY ACI 301.
 - DO NOT USE OR PLACE CONCRETE ON FROZEN MATERIALS OR MATERIALS CONTAINING ICE OR SNOW.
 - DO NOT USE CALCIUM CHLORIDE, SALT, OR OTHER MATERIALS CONTAINING ANTIFREEZE AGENTS OR CHEMICAL ACCELERATORS UNLESS APPROVED IN MIX DESIGNS.
 - PROTECT CONCRETE WORK FROM PHYSICAL DAMAGE OR REDUCED STRENGTH THAT COULD BE CAUSED BY FROST, FREEZING ACTIONS, OR LOW TEMPERATURES.
- HOT WEATHER CONCRETE PLACEMENT SHALL COMPLY WITH ACI 305.1 AND AS FOLLOWS:
 - MAINTAIN CONCRETE TEMPERATURE BELOW 95° F AT TIME OF PLACEMENT.
 - CHILLED MIXING WATER OR CHOPPED ICE MAY BE USED TO CONTROL TEMPERATURE, PROVIDED WATER EQUIVALENT OF ICE IS CALCULATED TO TOTAL AMOUNT OF MIXING WATER.

- BEFORE TEST SAMPLING AND PLACING OF CONCRETE, WATER MAY BE ADDED TO THE PROJECT SITE, SUBJECT TO THE LIMITATIONS OF ACI 301. DO NOT ADD WATER TO THE CONCRETE AFTER ADDING HIGH-RANGE WATER-REDUCING ADMIXTURES.
- SECURELY POSITION ALL ITEMS TO BE CAST IN PLACE SUCH AS REINFORCING DOWELS, ANCHORS, SLEEVES, ETC. PRIOR TO PLACEMENT OF CONCRETE.
- EMBEDDED CONDUITS, PIPES, AND SLEEVES SHALL MEET THE REQUIREMENTS OF ACI 318. REFERENCE TYPICAL DETAILS FOR ALLOWABLE PENETRATIONS AND ADDITIONAL REQUIRED REINFORCEMENT.

- PLACE ALL VERTICAL CONSTRUCTION JOINTS IN THE CENTER OF SPANS IN ACCORDANCE WITH THE TYPICAL DETAILS. CONTRACTOR SHALL SUBMIT PROPOSED LOCATIONS FOR CONSTRUCTION JOINTS NOT SHOWN ON STRUCTURAL DRAWINGS FOR REVIEW BY THE ARCHITECT AND ENGINEER.
- FOOTING, GRADE BEAM, AND SLAB AREAS SHALL BE CLEANED OF DEBRIS AND STANDING WATER PRIOR TO POURING CONCRETE.
- WHERE NOTED, SAW CUT JOINTS SHALL BE CUT AS SOON AS THE CONCRETE HAS OBTAINED ADEQUATE STRENGTH TO RESIST RAVELING OF THE JOINT EDGES, GENERALLY BETWEEN 4 TO 12 HOURS AFTER THE CONCRETE HAS BEEN FINISHED. HOWEVER, IF ENTRY IS DELAYED TOO LONG, SAWING CAN BECOME DIFFICULT AND UNCONTROLLABLE CRACKING MAY OCCUR. THE BEST TIME FOR SAWING SHALL BE DETERMINED IN THE FIELD AS TIMING MAY VARY BASED ON MIX DESIGN, PLACEMENT, AND CURING CONDITIONS. SAW CUTS SHALL BE A MINIMUM 1/4 OF THE SLAB THICKNESS, UNLESS NOTED OTHERWISE, WITH REINFORCEMENT CONTINUOUS THROUGH SAW CUTS IN ACCORDANCE WITH THE CONTROL JOINT DETAIL. DO NOT SAW CUT ELEVATED SLABS OR SLABS OVER VOID FORMS.

- CONCRETE SHALL REACH 70% OF THE SPECIFIED 28 DAY COMPRESSIVE STRENGTH AND BE IN PLACE FOR 7 DAYS PRIOR TO REMOVAL OF FORMS OR CONSTRUCTION ON TOP OF THE SLAB.

VOID FORMS AND SOIL RETAINERS

- ALL VOID FORM AND EARTH RETAINER WORK SHALL CONFORM TO THE FOLLOWING STANDARDS AND ANY STANDARDS REFERENCED THEREIN:
 - FFA-SC-11 - SPECIFICATION AND APPLICATION OF VOID SPACES BELOW CONCRETE FOUNDATIONS (FOUNDATION PERFORMANCE ASSOCIATION)
- VOID FORMS BELOW SLABS SHALL BE DEGRADABLE CORRUGATED PAPER (NON-COLLAPSIBLE). VOID FORMS BELOW BEAMS MAY BE DEGRADABLE CORRUGATED PAPER OR NON-DEGRADABLE COLLAPSIBLE.
- MINIMUM VOID DEPTH SHALL BE 8" IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
- MATERIALS AND PRODUCTS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:
 - FULLY WAX-IMPREGNATED CORRUGATED FIBERBOARD IS NOT ACCEPTABLE. IF THE PRODUCT EMPLOY'S PARTIAL WAX IMPREGNATION OR COATING, THE SOIL CONTACT AREA SHALL NOT BE FULLY WAX IMPREGNATED.
 - DO NOT USE POLYETHYLENE SHEATHING UNDER DEGRADABLE VOID FORMS.
 - NON-DEGRADABLE COLLAPSIBLE VOID FORMS
 - POLYPROPYLENE
 - SHALL BE CONSTRUCTED OF CORRUGATED PLASTIC, METAL, OR OTHER ACCEPTABLE PRODUCTS. CORRUGATED PAPER IS NOT ACCEPTABLE.
 - THE NET VOLUME OF THE COLLAPSED VOID FORM SHALL BE SUFFICIENT TO ACCEPT THE EXPANDING SOIL.
 - THE VOID FORM MATERIALS MUST COLLAPSE UNDER THE UPLIFT FORCES OF THE SOIL PER THE MAXIMUM COLLAPSE PRESSURES NOTED BELOW, BUT SHALL BE SUFFICIENTLY STIFF TO WITHSTAND THE WEIGHT OF THE CONCRETE AND ASSOCIATED CONSTRUCTION LOADS.
 - MAXIMUM COLLAPSE PRESSURES:
 - VOID FORMS UNDER SLABS: WEIGHT OF CONC. + 140 PSF
 - VOID FORMS UNDER GRADE BEAMS: WEIGHT OF CONF + 200 PSF
- SOIL RETAINERS
 - SHALL BE CONSTRUCTED OF HIGH DENSITY POLYETHYLENE OR HIGH DENSITY POLYPROPYLENE.
 - SHALL BE RIBBED TYPE RETAINERS AT EXTERIOR FACE OF GRADE BEAMS. RETAINERS AT INTERIOR FACE OF GRADE BEAMS SHALL BE CORRUGATED (NON-RIBBED) OR RIBBED TYPE. ALL SOIL RETAINERS SHALL BE CAPABLE OF PREVENTING SOIL MIGRATION TO THE VOIDS.
- PIER TOP FORMS
 - EITHER PRE-SCORED CORRUGATED PAPER/PLASTIC OR FIBER TUBE SHALL BE PROVIDED AT THE TOP 24" OF PIERS BELOW GRADE. ALTERNATELY, THE TOP 24" OF THE PIER MAY BE EARTH-FORMED WITH THE UNDERSTANDING THAT SUBSEQUENT EXCAVATION WILL TAKE PLACE AROUND THE TOP OF THE PIER SUCH THAT IT IS NOT IN CONTACT WITH THE SOILS.

- HANDLING AND INSTALLATION:
 - GENERAL
 - VOID FORMS AND SOIL RETAINERS SHALL BE STORED AND INSTALLED IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS.
 - STORE MATERIALS IN A MANNER THAT PROTECTS THEM FROM THE ELEMENTS.
 - DO NOT INSTALL OR USE MATERIALS THAT HAVE BEEN DAMAGED.
 - FORM WITH REMOVABLE OR SACRIFICIAL FORMS ON ALL SIDES OF ALL BELOW GRADE BEAMS, WALLS, PLASTERS, AND PIER CAPS. EARTH FORMING IS NOT PERMITTED. VOID FORMS ARE SACRIFICIAL AND SHALL NOT BE REUSED.
 - PROVIDE APPROPRIATE DRAINAGE WITHIN BEAM TRENCHES AND THROUGHOUT SLAB AREA TO ENSURE STANDING WATER DOES NOT PERSIST.
 - VAPOR RETARDER SHALL BE INSTALLED DIRECTLY IN CONTACT WITH CONCRETE AND SECURED TO CONCRETE IN ACCORDANCE WITH VAPOR RETARDER NOTES.
 - SEAM PADS, TAPE JOINTS, OR PROTECTIVE FIBERBOARD SHALL BE USED TO COVER OPEN JOINTS TO PREVENT CONCRETE INTRUSION.
 - VOID SPACE SYSTEMS SHALL BE PROPERLY PLACED AND ANCHORED TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT.
 - RAJIDUSED VOID FORMS SHALL BE PROVIDED ADJACENT TO PIERS AT THE INTERSECTION OF CONCRETE ABOVE.
 - ADDITIONAL REQUIREMENTS FOR DEGRADABLE VOID FORMS
 - DEGRADABLE VOID FORMS SHALL BE STORED IN A MANNER THAT PREVENTS THEM FROM BECOMING WET WHILE NOT ENTRAPPING HUMIDITY AND CONDENSATION.
 - THE USE OF DEGRADABLE VOID FORMS SHALL BE PLANNED TO MINIMIZE EXPOSURE TO ENVIRONMENTAL MOISTURE AND SHALL NOT BE LEFT IN PLACE ON GRADE FOR LONGER THAN (7) DAYS WITHOUT ADDITIONAL EVALUATION BY THE MANUFACTURER.
 - CONTRACTOR SHALL MAINTAIN A RAIN GAGE ON SITE. IF MORE THAN 0.2 INCHES OF RAIN FALLS WITHIN A 24 HOUR PERIOD AFTER FORMS ARE INSTALLED AND PRIOR TO CONCRETE PLACEMENT, SAID FORMS SHALL BE REMOVED AND REPLACED. IF STRUCTURAL INTEGRITY OF FORMS IS COMPROMISED BY ANY OTHER ISSUES, SAID FORMS SHALL BE REMOVED AND REPLACED. COST FOR REPLACEMENT OF FORMS SHALL BE THE BURDEN OF THE CONTRACTOR. NO CHANGE ORDERS OR SCHEDULE MODIFICATIONS ASSOCIATED WITH REPLACEMENT OF FORMS WILL BE ALLOWED.
 - PLACE A MINIMUM 1/4" COVER BOARD OVER DEGRADABLE VOID FORMS TO DISTRIBUTE WORKING LOAD, BRIDGE SMALL GAPS, AND PROTECT FORMS FROM PUNCTURE AND OTHER DAMAGES DURING REINFORCEMENT AND CONCRETE PLACEMENT.

- SOIL RETAINERS
 - PLACE SOIL RETAINERS AT SIDES OF VOID SPACE UNDER ALL GRADE BEAMS, WALLS, PLASTERS, AND PIER CAPS.
 - SOIL RETAINERS SHALL BE PROPERLY KEYED INTO THE UNDERLYING SOILS.
- CONSTRUCTION OF UNDER-SLAB UTILITIES SHALL BE SUCH THAT UTILITIES ARE NOT SOIL SUPPORTED AND HUNG FROM THE FOUNDATION ABOVE, UNLESS AN ALTERNATE DESIGN IS SUBMITTED AND APPROVED THAT ALLOWS FOR THE UTILITIES TO ADJUST TO THE POTENTIAL FOR DIFFERENTIAL MOVEMENT BETWEEN THE SUBGRADE AND THE FOUNDATION.
 - WHERE LINES TRANSITION FROM BEING HUNG AT THE INTERIOR TO SOIL SUPPORTED OUTSIDE OF THE BUILDING FOOTPRINT, PROVIDE A RETAINER CAP AT THE END OF THE VOID SPACE AND TRANSITION THE LINE INTO A 24" LONG SLEEVE, OVERSIZED TO 2X THE LINE DIAMETER. THE VOID SPACE BETWEEN LINE AND SLEEVE SHALL BE FILLED WITH NON-DEGRADABLE FOAM. THE OVERSIZED LINE SHALL BE SEALED AT THE END RETAINER CAP. ENTOMB THE OVERSIZED LINE WITH A MINIMUM 8" COVER OF FLOWABLE FILL.

SOIL RETAINERS

- FORM ALL SIDES OF PIER CAPS. EARTH FORMING IS NOT PERMITTED.
- PLACE SOIL RETAINERS AT SIDES OF VOID SPACE UNDER ALL STRUCTURAL CONCRETE PIER CAPS.
- INSTALL SOIL RETAINERS IN STRICT ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- STORE RETAINERS FLAT AND PROTECTED FROM DIRECT SUNLIGHT TO AVOID WARPING.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING STANDARDS AND ANY STANDARDS REFERENCED THEREIN:
 - AISC 303 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES
 - AISC 360 - SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS
 - AISC 341 - SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS
 - RCSC - SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS
 - AWS D1.1 - STRUCTURAL WELDING CODE - STEEL
- MATERIALS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS, UNLESS NOTED OTHERWISE:
 - W & WT SHAPES - ASTM A992, GRADE 50
 - C, S, & ST SHAPES - ASTM A36
 - ANGLE SHAPES - ASTM A36
 - STRUCTURAL PIPES - ASTM A53, GRADE B
 - ROUND HSS SHAPES - ASTM A500, GRADE C
 - SQUARE & RECTANGULAR HSS SHAPES - ASTM A500, GRADE C
 - STRUCTURAL PLATE & BARS - ASTM A36 OR ASTM A572 GRADE 50 AS NOTED
 - HIGH STRENGTH BOLTS - ASTM A325 (MECHANICALLY GALVANIZED) OR ASTM A490 (BLACK) ASTM A563 (MECHANICALLY GALVANIZED)
 - NUTS - ASTM F436
 - HARDENED STEEL WASHERS - ASTM F959
 - COMPRESSIBLE STEEL WASHERS - AWS D11 CLAUSE 7 TYPE B
 - HEADED CONCRETE ANCHORS - ASTM A1064
 - DEFORMED BARR ANCHORS - ASTM F1554, GRADE 36
 - ANCHOR RODS (STANDARD) - ASTM F1554, GRADE 55, WELDABLE
 - ANCHOR RODS (HIGH STRENGTH) - ASTM A36
 - THREADED RODS - AWS CLASS E70XX
 - WELDING ELECTRODES -
- ENDS OF COLUMN AT SPLICES AND AT OTHER BEARING CONNECTIONS SHALL BE "FINISHED TO BEAR" TO COMPLETE TRUE BEARING.
 - FABRICATE AND ASSEMBLE STRUCTURAL MEMBERS/ASSEMBLIES IN SHOP TO GREATEST EXTENT POSSIBLE.
 - ENDS OF COLUMNS AT SPLICES AND AT OTHER BEARING CONNECTIONS SHALL BE "FINISHED TO BEAR" TO COMPLETE TRUE BEARING.
- DIMENSIONAL TOLERANCES OF FABRICATED STRUCTURAL STEEL SHALL CONFORM TO SECTION 6.4 OF THE AISC CODE OF STANDARD PRACTICE UNLESS NOTED OTHERWISE.
- FABRICATE AND ASSEMBLE STRUCTURAL MEMBERS/ASSEMBLIES IN SHOP TO GREATEST EXTENT POSSIBLE.
- PROVIDE STIFFENERS 'FINISHED TO BEAR' UNDER ALL LOAD CONCENTRATIONS ON SUPPORTING MEMBERS OVER COLUMNS, AND WHERE SHOWN ON DRAWINGS.
- WORKING POINTS FOR VERTICAL BRACING SHALL BE AT THE INTERSECTION OF THE COLUMN CENTERLINE AND THE BEAM CENTERLINE UNLESS NOTED OTHERWISE. WORKING POINTS FOR VERTICAL BRACING AT COLUMN BASE PLATE SHALL BE AT THE INTERSECTION OF COLUMN CENTERLINE AND THE TOP OF THE BASE PLATE.
- BEAMS SHALL BE CAMBERED UPWARD WHERE SHOWN ON THE CONTRACT DOCUMENTS, WHERE NO UPWARD CAMBER IS INDICATED, ANY MILL CAMBER SHALL BE DETAILED UPWARD IN THE BEAMS.
- ALL STRUCTURAL STEEL MEMBERS, ASSEMBLIES, AND HARDWARE SHALL BE PRIMED AND PAINTED FOR EXPOSURE TO WEATHER. REPAIR ANY MEMBERS WELDED OR DAMAGED AFTER PROVIDE APPROPRIATE DRAINAGE WITHIN ZINC RICH PAINT OR APPROVED ALTERNATE.
- ALL STRUCTURAL STEEL MEMBERS, ASSEMBLIES, AND HARDWARE SHALL BE SHIPPED WITH ONE COAT OF SHOP PRIMER EXCEPT THOSE MEMBERS THAT ARE GALVANIZED.
- ALL STRUCTURAL STEEL MEMBERS BELOW 20'-0" ABOVE FINISH FLOOR SHALL BE EXPOSED ARCHITECTURALLY, CATEGORY 1 (AESS 1). PREPARED IN ACCORDANCE WITH AISC 303-16, TABLE 10.1, VERIFY WITH ARCH.
- SPLICING OF STRUCTURAL STEEL MEMBERS WHERE NOT DETAILED ON THE CONTRACT DOCUMENTS IS PROHIBITED WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER AS TO LOCATION, TYPE OF SPLICE, AND CONNECTION TO BE MADE.
- DECK EDGE ANGLES SHALL BE CONTINUOUS AND SHALL BE SPLICED ONLY AT SUPPORTS. SPLICES SHALL BE BUTT WELDED TO DEVELOP FULL CAPACITY OF THE MEMBER.
- DO NOT CUT STRUCTURAL STEEL MEMBERS UNLESS SO INDICATED IN THE DRAWINGS OR AS REVIEWED BY THE ENGINEER.
- THE USE OF DEGRADABLE VOID FORMS SHALL BE TRUE AND PLUMB BEFORE CONNECTIONS ARE FINALLY BOLTED OR WELDED.
- COLUMN BASE PLATES, LEVELING PLATES, OR BEARING PLATES SHALL BE SET TO THE ELEVATION INDICATED ON THE STRUCTURAL DRAWINGS AND LEVELLED USING SHIMS OR JAM NUTS AND WASHERS ON ANCHOR BOLTS. BASE PLATES SHALL THEN BE GROUTED. GROUT SHALL BE WITH NON-SHRINK, NON-METALLIC GROUT WITH MINIMUM 7 DAY COMPRESSIVE STRENGTH EQUAL TO TWICE THE 28 DAY STRENGTH OF THE SUPPORTING CONCRETE. COMPLETE GROUT WORK BEFORE PLACING CONCRETE ON LEVELS ABOVE (WHERE APPLICABLE). ANCHOR BOLTS SHALL BE PRESET USING TEMPLATES OR SIMILAR METHODS. TIGHTEN ANCHOR BOLTS AFTER SUPPORTED MEMBERS HAVE BEEN POSITIONED AND PLUMBED. HOLE SIZES IN BASE PLATES SHALL BE OVERSIZED PER AISC SECTION J3.2.
- HEADED CONNECTION ANCHORS AND DEFORMED BAR ANCHORS SHALL BE AUTOMATICALLY END WELDED WITH SUITABLE STUD WELDING EQUIPMENT IN THE SHOP OR IN THE FIELD. WELDING SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER.

METAL ROOF DECK

- METAL ROOF DECK SHALL BE AS FOLLOWS:

LOCATION	GAGE	DECK TYPE	DECK DEPTH (IN.)	MIN. SHEET WIDTH (IN.)	MIN. I _x (IN. ⁴)	MIN. S _x (IN. ³)	MIN. S _y (IN. ³)
TYP. UNO	20	WR	3	24	0.794	0.434	0.463

 - S_x - POSITIVE SECTION MODULUS IN.³
 - S_y - NEGATIVE SECTION MODULUS IN.³
 - I_x - MOMENT OF INERTIA IN.⁴
- SHEET STEEL FOR GALVANIZED ROOF DECK AND ACCESSORIES SHALL CONFORM TO ASTM A653, STRUCTURAL STEEL, WITH A MINIMUM YIELD STRENGTH OF 33 KSI AND A MAXIMUM YIELD STRENGTH OF 80 KSI. GALVANIZING SHALL CONFORM TO ASTM A653 WITH A MINIMUM COATING OF G90 (TYPICAL AT ALL LOCATIONS) AS DEFINED IN ASTM A653.
- ROOF DECKS SHALL BE CONSTRUCTED IN A TWO OR THREE SPAN CONDITION.
- PLACE DECK PANELS ON STRUCTURAL SUPPORTS AND ADJUST TO FINAL POSITION WITH ENDS LAPPED OR BUTTED OVER THE SUPPORTS WITH A MINIMUM END BEARING OF 2 1/2".
- ROOF DECKS SHALL BE ATTACHED TO PERPENDICULAR SUPPORTS AS NOTED ON S002.
- ROOF DECKS SHALL BE ATTACHED TO PARALLEL FRAMING AND ACCESSORIES, INCLUDING PERIMETER DECK EDGE MEMBERS WITH TYPICAL FRAME FASTENERS AT A MAXIMUM SPACING OF 12" OC, AND AS NOTED ON S002.
- ALL OPENINGS IN METAL ROOF DECKS SHALL BE REINFORCED IN ACCORDANCE WITH THE STRUCTURAL DETAILS.
- MECHANICAL, ELECTRICAL, AND PLUMBING SYSTEMS SHALL NOT BE SUPPORTED BY THE METAL ROOF DECK.

STEEL CONNECTIONS:

- ALL STRUCTURAL STEEL CONNECTIONS SHALL CONFORM TO REQUIREMENTS DETAILED IN THE "STRUCTURAL STEEL" NOTES AND ALL FOLLOWING PARAMETERS.
- STRUCTURAL STEEL CONNECTIONS NOT FULLY DETAILED IN THE STRUCTURAL DOCUMENTS SHALL BE DESIGNED AND DETAILED BY THE CONTRACTOR AND THE STEEL FABRICATOR, UNDER THE DIRECT SUPERVISION OF A LICENSED PROFESSIONAL ENGINEER IN THE STATE IN WHICH THE PROJECT IS TO BE CONSTRUCTED.
- CONCEPTUAL CONNECTION DETAILS ARE SHOWN ON THE DRAWINGS AND ARE APPLICABLE TO ALL CONNECTIONS NOT DESIGNED AND FULLY DETAILED IN THE DRAWINGS. THEY ARE PROVIDED ONLY TO INDICATE THE CONNECTION TYPE REQUIRED AND MAY NOT FULLY REPRESENT THE COMPLEXITY OF THE CONNECTION AS REQUIRED BY THE FINAL CONNECTION DESIGN.
- ADDITIONAL CONNECTION ELEMENTS NOT SHOWN ON TYPICAL OR CONCEPTUAL DETAILS MAY BE REQUIRED BY THE FINAL CONNECTION DESIGN, SUCH AS STIFFENER PLATES, DOUBLER PLATES, SUPPLEMENTAL / REINFORCING PLATES, OR OTHER CONNECTION MATERIAL. THE FABRICATOR'S LICENSED PROFESSIONAL ENGINEER IS RESPONSIBLE FOR SPECIFICATION OF THESE ELEMENTS OR CONVEYANCE OF THESE ADDITIONAL REQUIREMENTS TO THE ENGINEER OF RECORD FOR SPECIFICATION.
- THE FABRICATOR'S LICENSED PROFESSIONAL ENGINEER IN RESPONSIBLE CHARGE OF THE CONNECTION DESIGN SHALL REVIEW AND CONFIRM IN WRITING THAT THE SHOP AND ERECTION DRAWINGS PROPERLY INCORPORATE THE CONNECTION DESIGNS PRIOR TO SUBMITTING THE CONNECTION CALCULATION AND SHOP DRAWINGS TO THE ENGINEER-OF-RECORD FOR REVIEW.
- SEALED CALCULATIONS FOR ALL CONNECTIONS DESIGNED BY THE FABRICATOR'S LICENSED PROFESSIONAL ENGINEER SHALL BE SUBMITTED FOR REVIEW.
- CONNECTION DESIGN SHALL BE RELATED TO AND/OR REFERENCE THE SUBMITTED SHOP DRAWINGS TO FACILITATE REVIEW.
- ALL REACTIONS (SHEARS, AXIAL FORCES, MOMENTS, ETC.) SHOWN ON THE STRUCTURAL DRAWINGS ARE FACTORED LOADS CONFORMING TO THE REQUIREMENTS OF AISC LOAD AND RESISTANCE FACTOR DESIGN (LRFD). LOAD AND RESISTANCE FACTOR DESIGN (LRFD) SHALL BE USED IN THE SELECTION, COMPLETION, AND DESIGN OF CONNECTIONS.
- THE FABRICATOR SHALL WORK TO PRIORITIZE BOLTED FIELD CONNECTIONS OVER FIELD WELDS, WHERE FEASIBLE.
- WHERE STIFFENER PLATES ARE INDICATED ON THE DRAWINGS, THEY ARE REQUIRED AND SHALL BE PROVIDED AS INDICATED, WHERE SIZES ARE NOT INDICATED, WEB STIFFENERS SHALL BE A MINIMUM 3/8 INCH THICK, OR THE THICKNESS OF THE BEAM WEB, WHICHEVER IS LARGER.
- PROVIDE A MINIMUM 1/4 INCH THICK CAP PLATE AT THE END OF ALL HOLLOW STEEL SECTIONS NOT OTHERWISE CONNECTED TO AN ABUTTING MEMBER, UNLESS SPECIFICALLY DETAILED OTHERWISE, WHERE EXPOSED TO VIEW, PLATES SHALL MATCH THE INSIDE DIMENSION OF THE HOLLOW SECTION AND BE WELDED ALL AROUND WITH A GROOVE WELD WITHIN THE END OF THE MEMBER. PROVIDE OPENINGS AS REQUIRED FOR ANY HOT DIP GALVANIZATION.
- SHEAR CONNECTIONS
 - WHERE INDICATED, SHEAR CONNECTIONS SHALL BE DESIGNED FOR THE FORCES INDICATED ON THE STRUCTURAL DRAWINGS. SHEARS ARE INDICATED AT BEAM ENDS AS "W K" AT THE GIVEN MEMBER END.
 - IF NOT INDICATED ON THE STRUCTURAL DRAWINGS, CONNECTIONS SHALL BE DESIGNED FOR 55 PERCENT OF THE TOTAL LOAD CAPACITY FOR THE BEAM SPAN SHOWN IN THE BEAM TABLES IN THE CURRENT EDITION OF THE AISC STEEL CONSTRUCTION MANUAL.
 - SHEAR CONNECTIONS SHALL BE DESIGNED AS BOLTED CONNECTIONS, UNLESS WELDED TAB CONNECTIONS ARE PREFERRED FOR CONSTRUCTIBILITY AND APPROVED OTHERWISE.
 - THE MINIMUM NUMBER OF ROWS OF BOLTS SHALL BE 1/6 OF THE BEAM DEPTH WITH ANY FRACTION TO BE ROUNDED TO THE NEXT HIGHEST MEMBER, WHERE CONSTRUCTIBILITY DICTATES SMALLER CONNECTIONS, ALTERNATIVES MAY BE PROPOSED AND WILL BE REVIEWED ACCORDINGLY.
- MOMENT CONNECTIONS
 - WHERE INDICATED, MOMENT CONNECTIONS SHALL BE DESIGNED FOR THE FORCES INDICATED ON THE STRUCTURAL DRAWINGS. MOMENTS ARE INDICATED AS "M -".
 - IF NOT INDICATED ON THE STRUCTURAL DRAWINGS, CONNECTIONS SHALL BE DESIGNED TO DEVELOP THE FULL FLEXURAL CAPACITY OF THE MEMBER.
 - MOMENT CONNECTIONS SHALL BE DESIGNED AS WELDED CONNECTIONS, UNLESS DETAILED OR APPROVED OTHERWISE.
 - AXIAL FORCES ON BEAMS
 - WHERE HORIZONTAL FORCES ARE INDICATED ON PLAN AS "H -", CONNECTIONS SHALL BE DESIGNED TO TRANSFER THE HORIZONTAL FORCE IN ADDITION TO ANY INDICATED SHEAR AND/OR MOMENT FORCES.
 - MAIN FRAME BRACE AND TRUSS CONNECTIONS:
 - CONNECTIONS SHALL BE WELDED UNLESS DETAILED OR APPROVED OTHERWISE.
 - CONNECTIONS SHALL BE DESIGNED FOR THE FORCES INDICATED ON THE STRUCTURAL DRAWINGS.
 - IF NOT INDICATED ON THE STRUCTURAL DRAWINGS, CONNECTIONS SHALL BE DESIGNED TO DEVELOP THE FULL TENSILE CAPACITY OF THE MEMBERS.
- STEEL-TO-STEEL BOLTED CONNECTIONS:
 - SHORT SLOTTED HOLES ARE PERMITTED PROVIDED HARDENED WASHERS ARE INSTALLED IN ACCORDANCE WITH AISC REQUIREMENTS. WHERE HORIZONTAL FORCES ARE SPECIFIED ON THE DRAWINGS, SHORT SLOTTED HOLES ARE NOT PERMITTED PARALLEL TO THE LOAD AXIS.
 - ALL BOLTS SHALL BE 1/2 INCH DIAMETER AND CONFORM TO ASTM A325, UNLESS NOTED OTHERWISE. BOLTS SHALL BE DESIGNED USING VALUES FOR BEARING TYPE BOLTS WITH THREADS ALLOWED IN THE SHEAR PLANE.
 - BOLTS SHALL BE TIGHTENED TO "SNUG TIGHT" AS DEFINED BY AISC, UNLESS NOTED OTHERWISE ON THE STRUCTURAL DOCUMENTS OR ON THE SEALED CONNECTION DESIGN SUBMITTAL.
 - BOLTS NOTED AS "SLIP CRITICAL" (SC) ON THE DOCUMENTS SHALL BE TIGHTENED TO THE MINIMUM PRE-TENSIONED AMOUNTS ACCORDING TO SECTION J OF THE AISC SPECIFICATION USING ONE OF THE FOLLOWING METHODS: TURN-OF-THE-NUT METHOD, CALIBRATED TORQUE WRENCH, TWIST-OFF TYPE TENSION CONTROL, OR DIRECT TENSION INDICATORS.
- WELDED CONNECTIONS:
 - ALL WELDING SHALL CONFORM TO ANSI/AWS D1.1, LATEST EDITION.
 - MINIMUM FILLET WELD SIZE SHALL BE 3/16 INCHES OR THAT REQUIRED BY AISC, WHICHEVER IS LARGER.
 - FOR CONNECTIONS NOT SPECIFIED BY THESE NOTES OR STRUCTURAL DRAWINGS, PROVIDE FILLET WELDS AT ALL CONTACT SURFACES SUFFICIENT TO DEVELOP THE FULL TENSILE STRENGTH OF THE SMALLEST MEMBER BEING JOINED.

COLD-FORMED METAL FRAMING

- ALL EXTERIOR AND LOAD-BEARING COLD-FORMED STRUCTURAL NOTED ON THESE PLANS OR DETAILED IN DESIGNATED SUBMITTALS SHALL CONFORM TO THE FOLLOWING STANDARDS AND ANY STANDARDS REFERENCED THEREIN:
 - AISI S100 - NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS
 - AISI S200 - NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING
 - AWS D1.3 - STRUCTURAL WELDING CODE - SHEET STEEL
- MATERIALS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS, UNLESS NOTED OTHERWISE:
 - STEEL SHEET - ASTM A1003, TYPE H
 - FRAMING MEMBERS W/ THICKNESS ≤ 0.0428" (18 GA) - 33 KSI
 - FRAMING MEMBERS W/ THICKNESS ≥ 0.0538" (16 GA) - 50 KSI
 - CLIPS AND CONNECTORS - 50 KSI
 - STEEL TAPPING SCREWS - ASTM C1513
- COLD-FORMED STRUCTURAL FRAMING SIZES SHALL BE AS NOTED ON PLANS WITH THE FOLLOWING PROPERTIES:

DESIGNATION THICKNESS (MILS)	THICKNESS (GAUGE)	MINIMUM THICKNESS (INCHES)	STEEL GRADE (KSI)
33	20	0.0346	33
43	18	0.0451	
54	16	0.0566	50
68	14	0.0677	
97	12	0.0966	

- COLD FORMED WALL ASSEMBLIES SHALL CONSIST OF THE FOLLOWING COMPONENTS:
 - STUDS - NOT TO EXCEED 16" ON CENTER, W/ 1 1/2" X 4" PUNCHOUTS 12" FROM EACH END AND AT 24" ON CENTER.
 - BOTTOM TRACKS - SIZE AND GAUGE TO MATCH STUDS WITH A MINIMUM 1 1/4" FLANGE OR AS NOTED ON DRAWINGS. PROVIDE AT BOTTOM OF WALLS.
 - TOP TRACKS - SIZE AND GAUGE TO MATCH STUDS WITH A MINIMUM 1 1/4" FLANGE OR AS NOTED ON DRAWINGS. PROVIDE AT TOP OF PARAPET WALLS AND BELOW OPENINGS.
 - DEFLECTION TRACKS - DEPTH TO MATCH STUD SIZE. MINIMUM 2 1/2" FLANGE W/ 1 1/2" SLOT. PROVIDE AT TOPS OF WALLS WHERE ATTACHING TO STEEL STRUCTURE ABOVE.
 - BY-PASS DEFLECTION CLIPS - MINIMUM 2 1/4" VERTICAL SLOTS, ANCHORED TO DECK CLOSEOUT. PROVIDE AT EACH BY-PASS STUD AT EACH FLOOR AND ROOF LEVEL.
 - RIGID CLIPS - ANCHORED TO STEEL FRAMING. PROVIDE AT EACH STUD WHERE TOP TRACKS ARE UNSUPPORTED.
- TOP AND BOTTOM TRACKS SHALL BE MATCHING SIZE AND GAUGE AS STUD WALLS, WITH MIN. A K" AT FLANGE, UNLESS NOTED OTHERWISE.
- COLD-FORMED STRUCTURAL FRAMING SHALL HAVE MINIMUM PROTECTION COATING EQUAL TO G-60 GALVANIZED FINIS
- STRUCTURAL FRAMING SHALL BE PROPERLY SPACED, PLUMBED, LEVELLED, SQUARED, FIT PROPERLY AGAINST ABUTTING MEMBERS, AND HELD SECURELY IN PLACE UNTIL PERMANENTLY FASTENED. WIRE TYING OF STRUCTURAL FRAMING COMPONENTS IS NOT PERMITTED.
- FASTENING OF COLD-FORMED STRUCTURAL FRAMING SHALL CONFORM TO THE FOLLOWING METHODS:
 - SHEET STEEL TO SHEET STEEL - STEEL TAPPING SCREWS
 - SHEET STEEL TO CONCRETE - POWDER ACTUATED FASTENERS
 - SHEET STEEL TO HOT-ROLLED STEEL (≥ 3/16" THICKNESS) - STEEL TAPPING SCREWS OR POWDER ACTUATED FASTENERS
- DO NOT WELD SHEET STEEL UNLESS SPECIFICALLY NOTED ON THE DRAWINGS. PROTECTIVE COATING REMOVED BY WELDING SHALL HAVE THE COATING REPAIRED AT THE WELDS BY PAINTING WITH A ZINC RICH PRIMER.
- COLD-FORMED STRUCTURAL FRAMING MAY BE SHOP OR FIELD FABRICATED INTO ASSEMBLIES, PRIOR TO ERECTION, OR ASSEMBLED IN THE FIELD.
- THE STRUCTURAL FRAMING SHALL HAVE ENDS SQUARELY CUT BY SHEARING OR SAWING, BE INSTALLED PLUMB, SQUARE, TRUE TO LINE, AND SECURELY FASTENED PER THE CONTRACT DOCUMENTS OR APPROVED CONNECTION DETAILS. TORCH CUTTING IS NOT PERMITTED.
- SPLICING OR NOTCHING OF FRAMING MEMBERS IS NOT PERMITTED UNLESS EXPLICITLY NOTED ON THE DRAWINGS.
- PROVIDE JAMB STUDS, BOX HEADERS, AND SILL TRACKS AROUND OPENINGS.
- TEMPORARY BRACING OF THE STRUCTURAL FRAMING SHALL BE PROVIDED AS REQUIRED AND REMOVED ONLY AFTER THE FRAMING HAS BEEN SECURED WITH PERMANENT SUPPORT.
- FRAMING MEMBERS SHALL BE LINED UP DIRECTLY OVER VERTICAL SUPPORTS OR BE SUPPORTED BY A LOAD DISTRIBUTION MEMBER PER THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS. THE TOP TRACK IS NOT PERMITTED TO ACT AS A DISTRIBUTION MEMBER UNLESS DESIGNED FOR THAT CONDITION.
- FRAMING MEMBERS IN BEARING CONDITIONS SHALL HAVE A MINIMUM 1 1/2" OF BEARING LENGTH, A MINIMUM 10' OF UNPUNCHED WEB FROM AN END SUPPORT AND MINIMUM 10' OF UNPUNCHED WEB ON EITHER SIDE OF AN INSIDE SUPPORT.
- STRUCTURAL FRAMING WEB STIFFENERS SHALL BE LOCATED AND INSTALLED PER THE CONTRACT DOCUMENTS OR APPROVED SHOP DRAWINGS.
- PROVIDE END BLOCKING OR A CONTINUOUS TRACK WHERE JOIST ENDS ARE NOT RESTRAINED AGAINST ROTATION.
- REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL DETAILS, DIMENSIONS, AND LOCATIONS OF FRAMING.

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ISSUED SETS

Date	Description
05/01/2023	Bid Set

REVISIONS

No.	Date	Description

Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

SHEET NAME

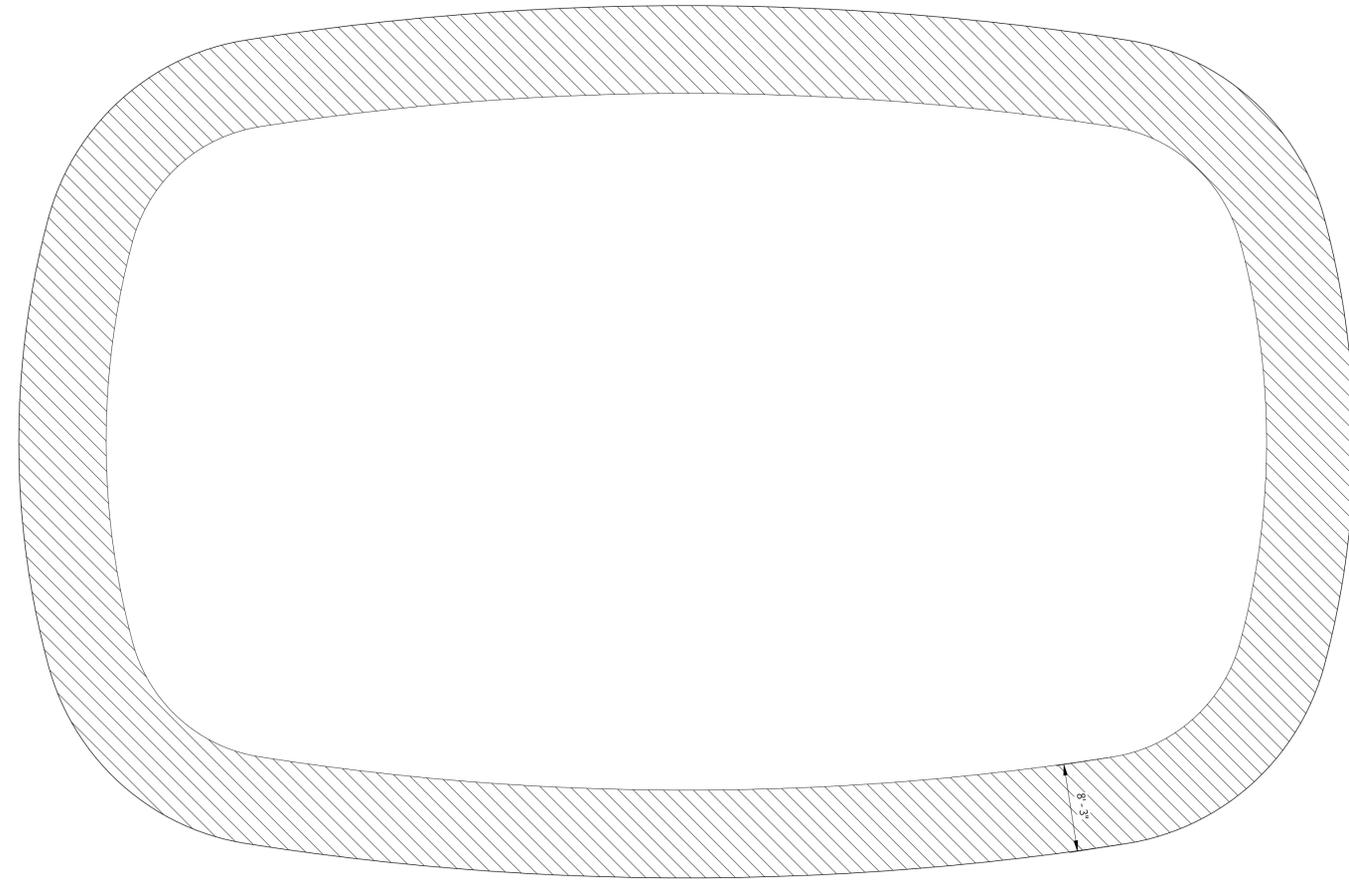
NOTES

Drawn by: IFW	Phase	BID SET
Checked by: NAG		
GVI Approval	GVI Job Number	22118
Client Approval	GE Job Number	22-1291
Date	05/01/23	

SCALE AS NOTED

SHEET NUMBER

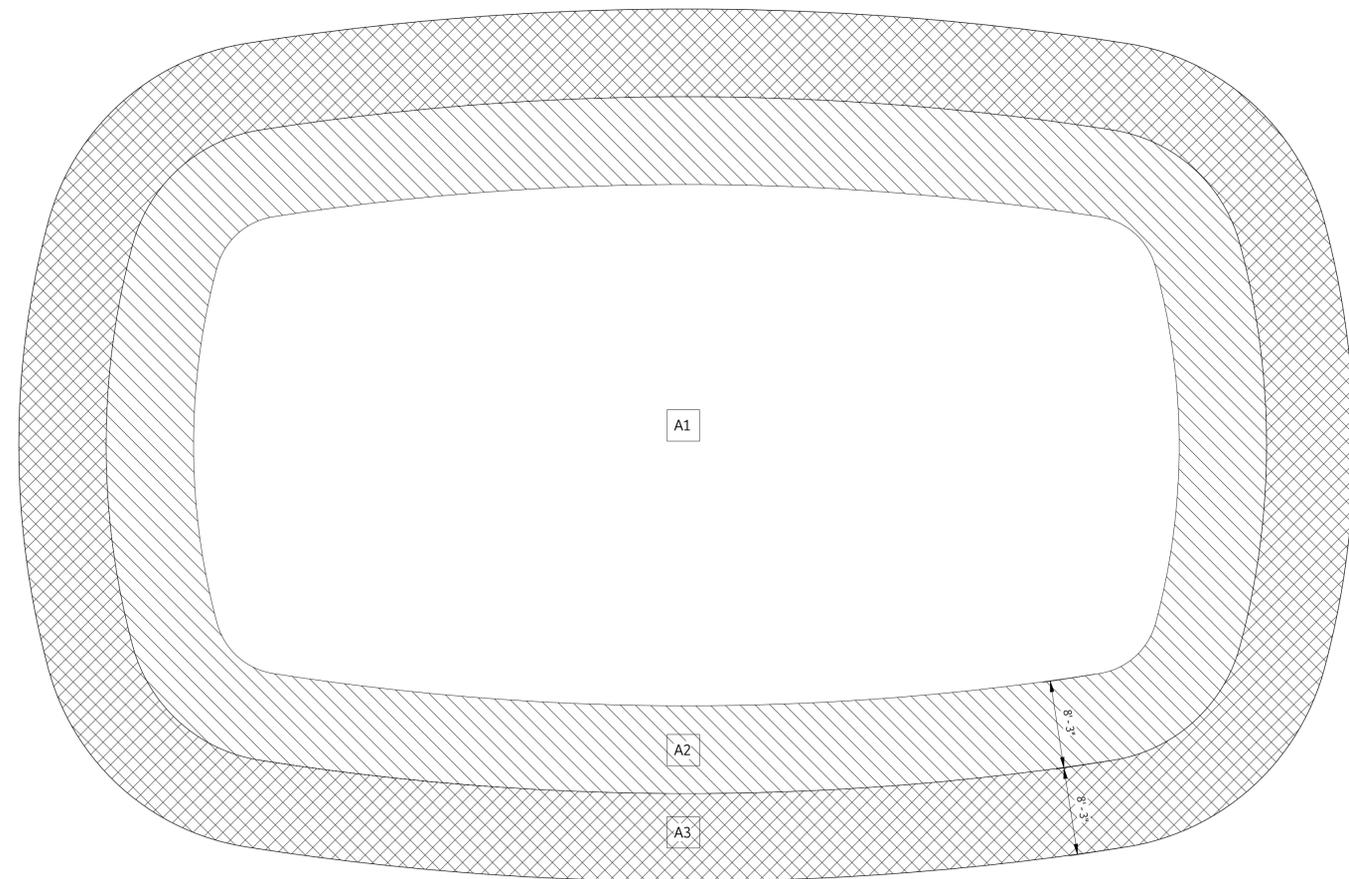
S001



① ROOF DECK CONNECTION PLAN
1/8" = 1'-0"

ROOF CONNECTION TABLE		
HATCH	FASTENING PATTERN 1	SIDELAP CONNECTOR SPACING 2
	32/3	36" OC
	32/5	36" OC

NOTES:
 1. FASTEN DECK AT END LAPS, END SUPPORTS, AND INTERMEDIATE SUPPORTS USING 5/8"Ø PUDDLE WELDS OR HILTI X-EMP19 POWDER ACTUATED FASTENERS, SPACED PER TABLE ABOVE.
 2. FASTEN SIDELAPS OF ADJACENT ROOF DECK UNITS USING #10-16 SELF DRILLING SCREWS, SPACED PER TABLE ABOVE.



② ROOF UPLIFT PRESSURE PLAN
1/8" = 1'-0"

ROOF WIND PRESSURES			
ZONE	WIND PRESSURE BASED ON TRIBUTARY AREA (PSE)		
	<67 SF	>=67 SF <269 SF	>269 SF
A1	+19.1/-17.1	+19.1/-17.1	+19.1/-17.1
A2	+28.6/-26.7	+28.6/-26.7	+19.1/-17.1
A3	+38.2/-52.0	+28.6/-26.7	+19.1/-17.1

NOTES:
 1. ALL NOTED PRESSURES SHOWN ARE STRENGTH LEVEL (LRF) UPLIFT PRESSURES APPLIED TO THE TOP OF THE ROOF SURFACE.

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**Hemisfair Sports
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PROFESSIONAL SEAL

SHEET NAME
**ROOF CONNECTIONS AND DESIGN
 LOAD PLANS**

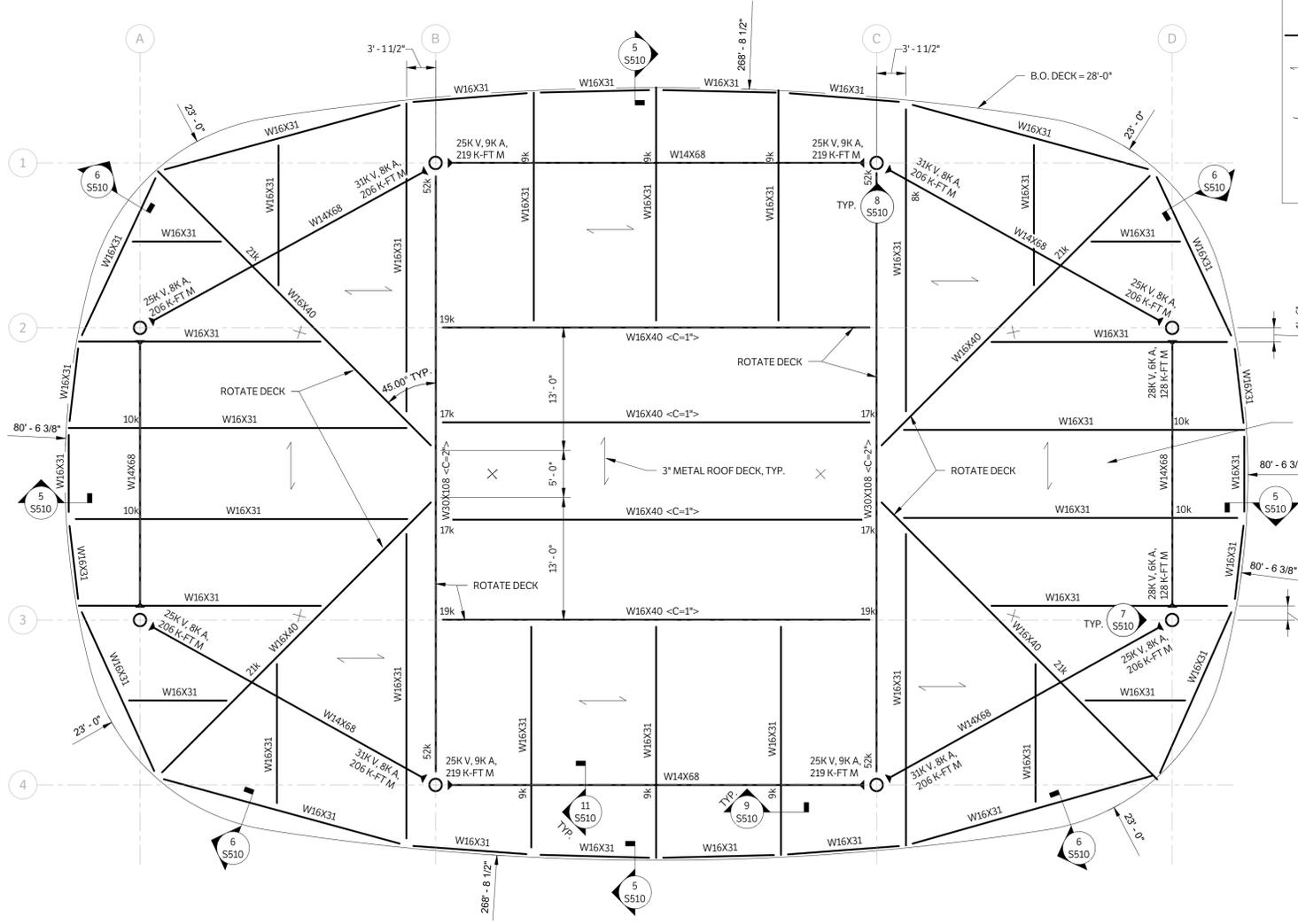
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GVI Approval	Client Approval	GE Job Number 22-1291
Date	05/01/23	

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S002

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NOTES:

1. ALL MEMBERS SHALL BE TOP JUSTIFIED WITH BOTTOM OF DECK, EXCEPT MOMENT W14 GIRDERS AROUND THE COLUMN RING, WHICH SHOULD BE SET BELOW OVER-PASSING JOISTS
2. ELEVATIONS ARE BASED ON FFE = 0'-0"
3. STEEL BEAMS AND JOISTS ARE CENTERED ON AND EQUALLY SPACED BETWEEN COLUMN CENTERLINES, UNLESS NOTED OTHERWISE.
4. ALL BEAM CONNECTIONS SHALL BE DESIGNED FOR REACTIONS SHOWN AT BEAM SUPPORTS, OR 8K MIN. REACTION IF END REACTION NOT SHOWN.
5. ALL BEAM TO BEAM AND BEAM TO COLUMN CONNECTIONS SHALL BE SINGLE PLATE OR DOUBLE ANGLE SHEAR CONNECTIONS, UNLESS NOTED OTHERWISE.
6. COORDINATE AND VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
7. REFERENCE GENERAL NOTES, TYPICAL DETAILS, AND PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION.
8. REFERENCE THE FOLLOWING SHEETS FOR TYPICAL DETAILS AND SCHEDULES:
 - S-510 TYPICAL STEEL DETAILS

LEGEND:

- W16X40 c=3/4" 31k ← BEAM CAMBER
- ← BEAM SHEAR REACTION
- ← SHEAR
- ← AXIAL
- ← MOMENT
- ← MOMENT CONNECTION ON BEAM, REF. 7 / S510, 8 / S510
- ← DIRECTION OF DECK SPAN
- ← RADIUS OF CURVE

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1 ROOF FRAMING PLAN
 1/8" = 1'-0"

BID SET

PROFESSIONAL SEAL

SHEET NAME
 ROOF PLAN

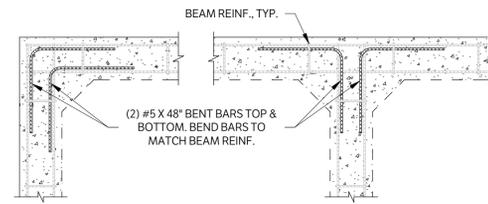
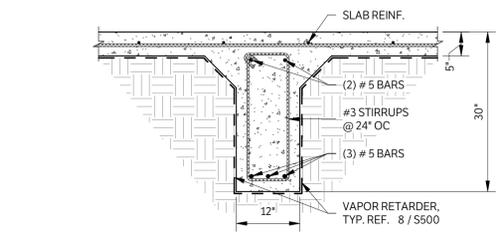
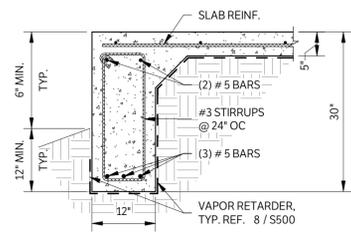
Drawn by: EL	Phase
Checked by: NAG	BID SET
GVI Approval	GVI Job Number 22118
Client Approval	GE Job Number 22-1291
Date	09/14/18

SCALE AS NOTED
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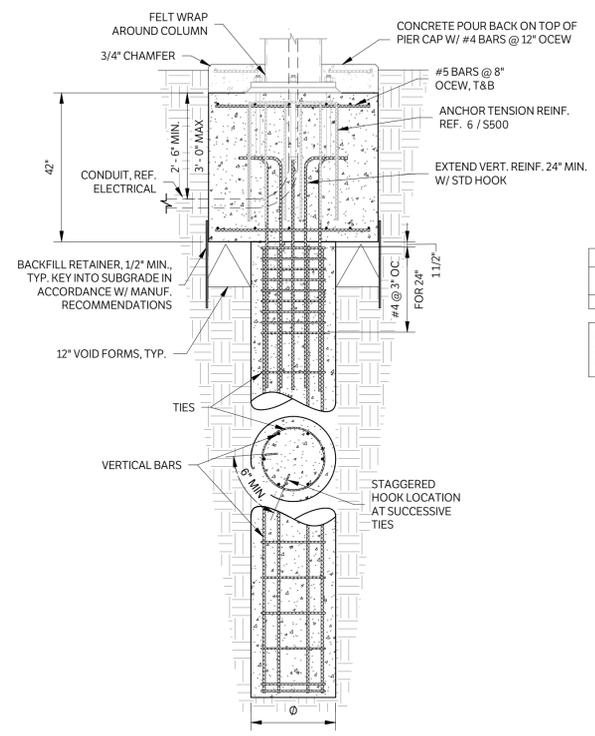
S110

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- SHEET NOTES**
- REFERENCE PLANS FOR SLOPING SLAB CONDITIONS.
 - WHERE SLAB DROP IS LESS THAN OR EQUAL TO 1/2", SLAB REINFORCEMENT MAY BE CONTINUOUS AND BENT BELOW DROP IN LIEU OF PROVIDING BENT BARS AS SHOWN.
 - VAPOR RETARDER AS DETAILED TO BE INSTALLED BELOW ALL FOUNDATION CONCRETE.
 - ALL SPLICES SHALL BE AS SPECIFIED IN THE GENERAL NOTES.
 - BEAM REINFORCEMENT TYPICAL UNLESS NOTED OTHERWISE.
 - BEAM DEPTH AND WIDTH TYPICAL UNLESS NOTED OTHERWISE.



- NOTES:**
- PROVIDE CORNER BARS AS SHOWN AT ALL EXTERIOR BEAM INTERSECTIONS.
 - THIS IS A SCHEMATIC ONLY. SEE BEAM SECTIONS FOR ACTUAL BEAM REINFORCEMENT. (SLAB NOT SHOWN)



PIER TABLE		
PIER Ø	VERTICAL REINF.	TIE REINF. & SPACING
30"	(9) #8 BARS	#3 TIES @ 16" OC

NOTE: BEAR PIERS 35' BELOW EXISTING GROUND SURFACE, REF. GEOTECHNICAL REPORT.

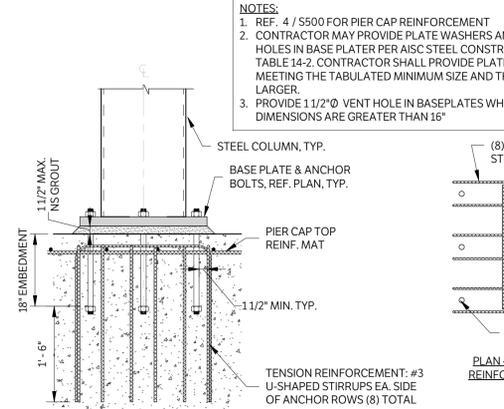
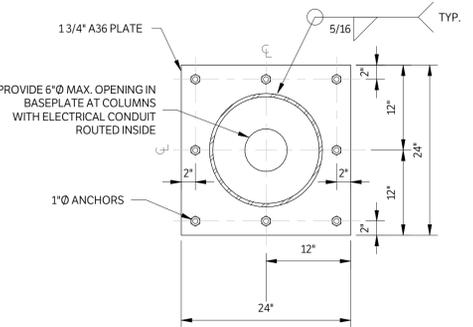
1 EXTERIOR BEAM N.T.S.

2 INTERIOR BEAM N.T.S.

3 GRADE BEAM INTERSECTIONS N.T.S.

4 STRAIGHT SHAFT PIER & PIER CAP DETAIL N.T.S.

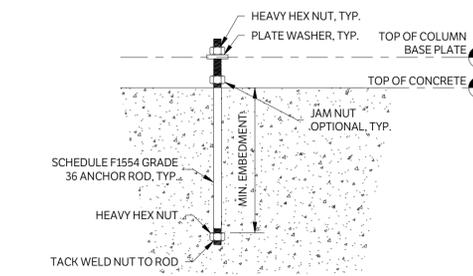
NOTE: REF. DETAIL 6 / S500 FOR BASE PLATE ELEVATIONS.



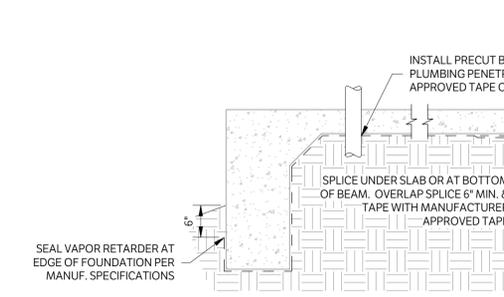
5 BASE PLATE - BP1 N.T.S.

6 BASE PLATE ELEVATION HSS N.T.S.

- NOTES:**
- REFER TO 5 / S500 ANCHOR ROD SIZE & EMBEDMENT.
 - ANCHOR ROD NUTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED BY AISC AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD, UNO.
 - THE HOLE IN THE PLATE WASHER SHALL BE 1/16" LARGER THAN THE ROD DIAMETER.



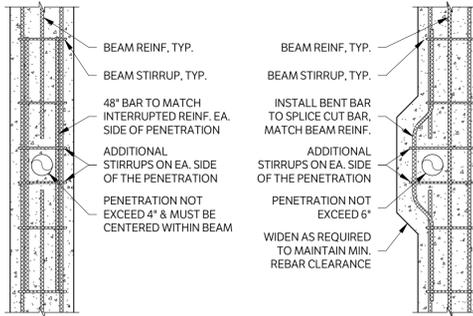
NOTE: VAPOR RETARDER AS DETAILED TO BE INSTALLED BELOW ALL FOUNDATION CONCRETE



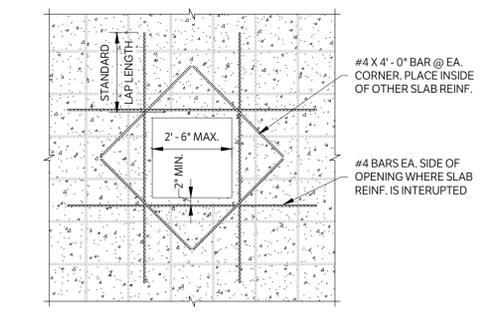
7 ANCHOR BOLT TYPES N.T.S.

8 VAPOR RETARDER DETAIL N.T.S.

BEAM PENETRATION $\le 4''$ CENTERED WITHIN BEAM



BEAM PENETRATION $\le 6''$ AND/OR OFF CENTER OF BEAM



10 BEAM PENETRATION VERTICAL N.T.S.

11 SLAB OPENING REINF. N.T.S.

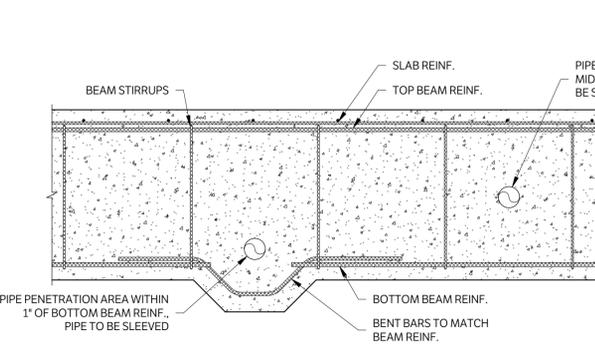
STANDARD DOWEL HOOK DIMENSIONS ALL GRADES OF STEEL

BAR SIZE	Ø	90°		180°	
		A	H	A	H
#3	2 1/4"	6"	5"	4"	
#4	3"	8"	6"	4 1/2"	
#5	3 3/4"	10"	7"	5"	
#6	4 1/2"	12"	8"	6"	
#7	5 1/4"	14"	10"	7"	
#8	6"	16"	11"	8"	

NOTE: Ø = FINISHED INSIDE BEND

12 STANDARD DOWEL HOOKS N.T.S.

NOTE: WHEN PIPE SLEEVE IS WITHIN 1' OF BEAM REINF. BEAM REINF. SHALL BE STOPPED AND BENT BARS ADDED AS SHOWN BELOW.



9 BEAM PENETRATION DETAIL N.T.S.

STIRRUP/TIE HOOK DIMENSIONS ALL GRADES OF STEEL

BAR SIZE	Ø	90°		135°	
		A	H	A	H
#3	1 1/2"	4"	4 1/4"	3"	
#4	2"	4 1/2"	4 1/2"	3"	
#5	2 1/2"	6"	5 1/2"	3 3/4"	
#6	4 1/2"	12"	8"	4 1/2"	
#7	5 1/4"	14"	9"	5 1/4"	
#8	6"	16"	10 1/2"	6"	

NOTE: Ø = FINISHED INSIDE BEND

13 STIRRUP/TIE HOOKS N.T.S.

GESSNER ENGINEERING
 401 W. 26th Street
 Bryan, TX 77802
 Tx Registered Engineering Firm F-7451

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ISSUED SETS

Date	Description
05/01/2023	Bid Set

REVISIONS

No.	Date	Description
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Hemisfair Sports Court Pavilion

Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

SHEET NAME
 FOUNDATION DETAILS

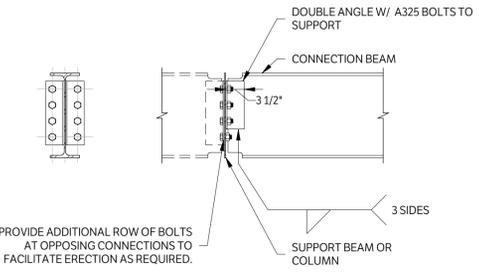
Drawn by: IFW	Phase	BID SET
Checked by: NAG		
GVI Approval	GVI Job Number	22118
Client Approval	GE Job Number	22-1291
Date	05/01/23	

SCALE AS NOTED
 SHEET NUMBER

BID SET

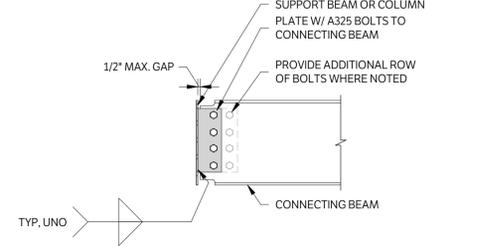
S500

NOTES:
 1. PROVIDE THIS CONNECTION WHERE NOTED AND FOR THE FOLLOWING CONDITIONS:
 A. WIDE FLANGE BEAM TO WIDE FLANGE BEAM CONNECTIONS.
 B. WIDE FLANGE BEAM TO WIDE FLANGE COLUMN.
 2. A SINGLE PLATE SHEAR CONNECTION PER 2 / S510 MAY BE USED IN LIEU OF DOUBLE ANGLES WHERE CAPACITIES ARE SUFFICIENT.



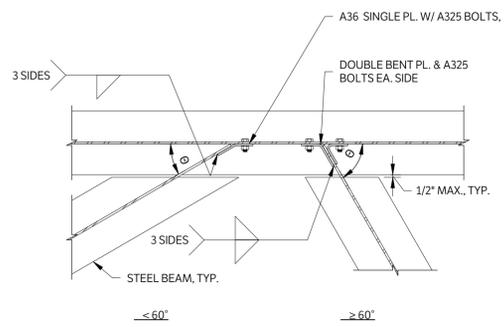
① SHEAR CONNECTION - DOUBLE ANGLE
N.T.S.

NOTES:
 1. PROVIDE THIS CONNECTION WHERE NOTED AND FOR THE FOLLOWING CONDITIONS:
 A. WIDE FLANGE BEAM TO HSS COLUMN.
 B. WIDE FLANGE BEAM TO BEAM OR COLUMN CONNECTIONS WHERE CAPACITIES ARE SUFFICIENT.

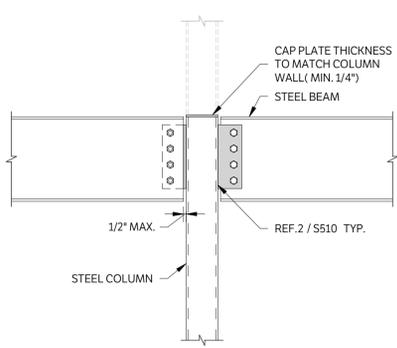


② SHEAR CONNECTION - SINGLE PLATE
N.T.S.

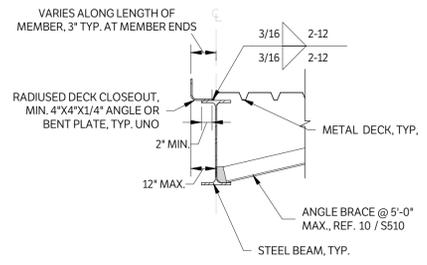
NOTE: PROVIDE THIS CONNECTION AT SKEWED BEAM TO BEAM CONNECTIONS



③ SKEWED BEAM TO BEAM CONNECTIONS
N.T.S.

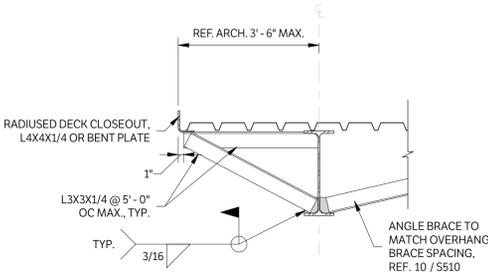


④ BEAM TO HSS COLUMN SHEAR CONNECTION
N.T.S.

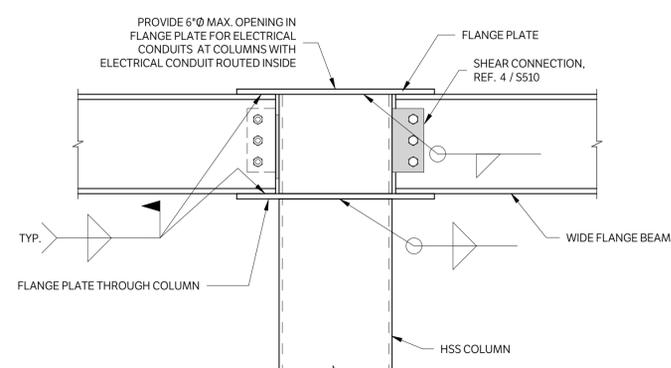


⑤ TYPICAL DECK CLOSEOUT AT ROOF
N.T.S.

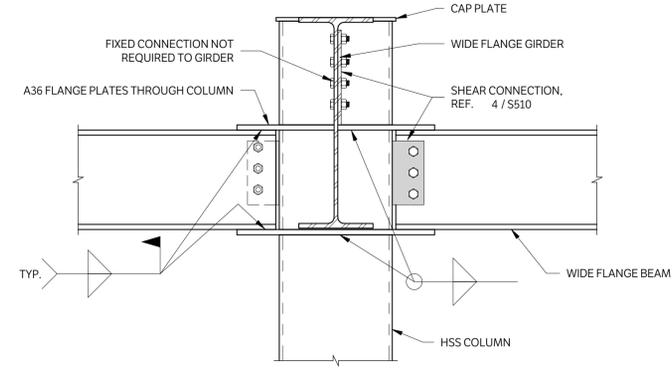
NOTE:
 1. DECK MAY BE PARALLEL (AS SHOWN) OR PERPENDICULAR TO BEAM.
 2. WHERE DECK CLOSEOUT CANNOT BE BENT TO MATCH RADIUS, CLOSEOUT MAY BE SPLICED. SPLICES SHOULD OCCUR OVER BRACE LOCATIONS, AND SHALL BE FULLY WELDED TOGETHER.



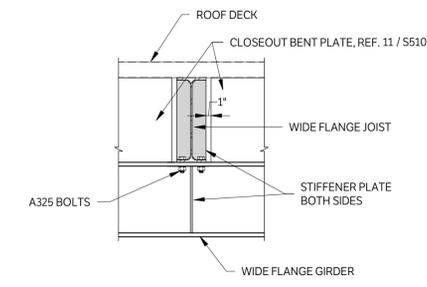
⑥ ROOF DECK LONG OVERHANG
N.T.S.



⑦ BEAM TO HSS COLUMN MOMENT CONNECTION
N.T.S.



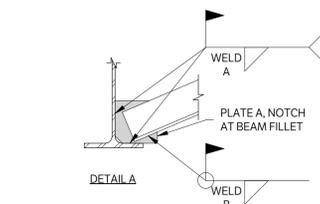
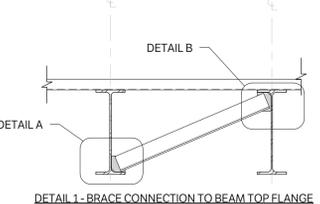
⑧ BEAM TO HSS COLUMN MOMENT CONNECTION W/ PERP. GIRDER
N.T.S.



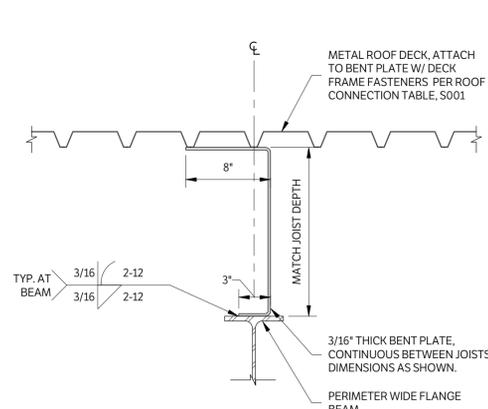
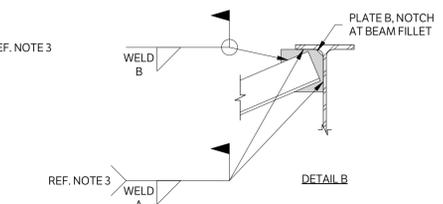
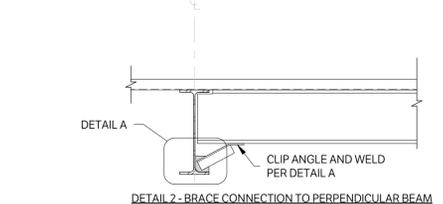
⑨ WIDE FLANGE JOIST OVER GIRDER
N.T.S.

CONNECTION SCHEDULE				
BRACE SIZE	PLATES A&B	WELDS A&B	CONNECTION ANGLE	
L2X2X1/4	4X4X1/4	3/16	L3X3X1/4	
L3X3X1/4	4X4X1/4	3/16	L4X4X1/4	
L4X4X1/4	4X4X1/4	3/16	L5X5X5/16	
L5X5X5/16	4X4X1/4	1/4	L6X6X1/2	

NOTES:
 1. REFERENCE PLANS AND DETAILS FOR LOCATION OF ANGLE BRACES.
 2. IF NOT SHOWN ON PLANS OR DETAILS, BRACE SIZE SHALL BE L3X3X1/4.
 3. FILED WELDING OF CONNECTION PLATES SHALL BE FABRICATOR'S OPTION.



⑩ BEAM BRACING
N.T.S.



⑪ CLOSEOUT BENT PLATE
N.T.S.

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 401 W. 26th Street
 Bryan, TX 77802
 TX Registered Engineering Firm F-7451

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ISSUED SETS

Date	Description
05/01/2023	Bid Set

REVISIONS

No.	Date	Description
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Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

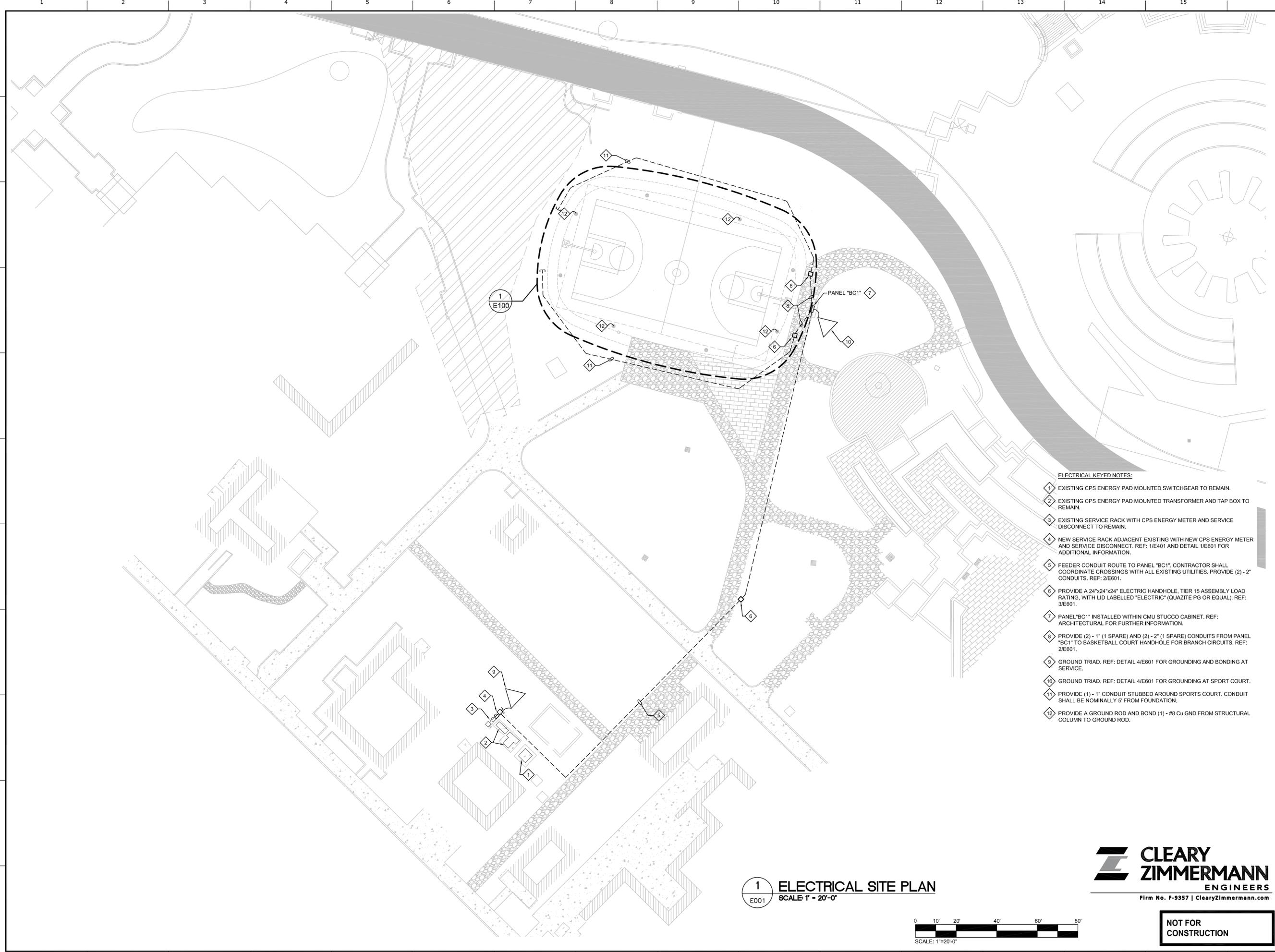
SHEET NAME
 STEEL FRAMING DETAILS

Drawn by: JFW	Phase	BID SET
Checked by: NAG	GVI Job Number	22118
GVI Approval	GE Job Number	22-1291
Client Approval	Date	05/01/23

SCALE AS NOTED
SHEET NUMBER

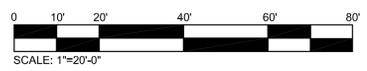
S510

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- ELECTRICAL KEYED NOTES:**
- 1 EXISTING CPS ENERGY PAD MOUNTED SWITCHGEAR TO REMAIN.
 - 2 EXISTING CPS ENERGY PAD MOUNTED TRANSFORMER AND TAP BOX TO REMAIN.
 - 3 EXISTING SERVICE RACK WITH CPS ENERGY METER AND SERVICE DISCONNECT TO REMAIN.
 - 4 NEW SERVICE RACK ADJACENT EXISTING WITH NEW CPS ENERGY METER AND SERVICE DISCONNECT. REF: 1/E401 AND DETAIL 1/E601 FOR ADDITIONAL INFORMATION.
 - 5 FEEDER CONDUIT ROUTE TO PANEL "BC1". CONTRACTOR SHALL COORDINATE CROSSINGS WITH ALL EXISTING UTILITIES. PROVIDE (2) - 2" CONDUITS. REF: 2/E601.
 - 6 PROVIDE A 24"x24"x24" ELECTRIC HANDHOLE, TIER 15 ASSEMBLY LOAD RATING, WITH LID LABELLED "ELECTRIC" (QUAZITE PG OR EQUAL). REF: 3/E601.
 - 7 PANEL "BC1" INSTALLED WITHIN CMU STUCCO CABINET. REF: ARCHITECTURAL FOR FURTHER INFORMATION.
 - 8 PROVIDE (2) - 1" (1 SPARE) AND (2) - 2" (1 SPARE) CONDUITS FROM PANEL "BC1" TO BASKETBALL COURT HANDHOLE FOR BRANCH CIRCUITS. REF: 2/E601.
 - 9 GROUND TRIAD. REF: DETAIL 4/E601 FOR GROUNDING AND BONDING AT SERVICE.
 - 10 GROUND TRIAD. REF: DETAIL 4/E601 FOR GROUNDING AT SPORT COURT.
 - 11 PROVIDE (1) - 1" CONDUIT STUBBED AROUND SPORTS COURT. CONDUIT SHALL BE NOMINALLY 5' FROM FOUNDATION.
 - 12 PROVIDE A GROUND ROD AND BOND (1) - #8 Cu GND FROM STRUCTURAL COLUMN TO GROUND ROD.

1 ELECTRICAL SITE PLAN
 E001 SCALE: 1" = 20'-0"



CLEARY ZIMMERMANN ENGINEERS
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ISSUED SETS

Date	Description

REVISIONS

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

PRELIMINARY REVIEW

NOT FOR REGULATORY APPROVAL, PERMIT OR CONSTRUCTION

AARON T. LOVELOCK
 P.E. REG. NO. 145763
MAY 1, 2023

SHEET NAME

ELECTRICAL SITE PLAN

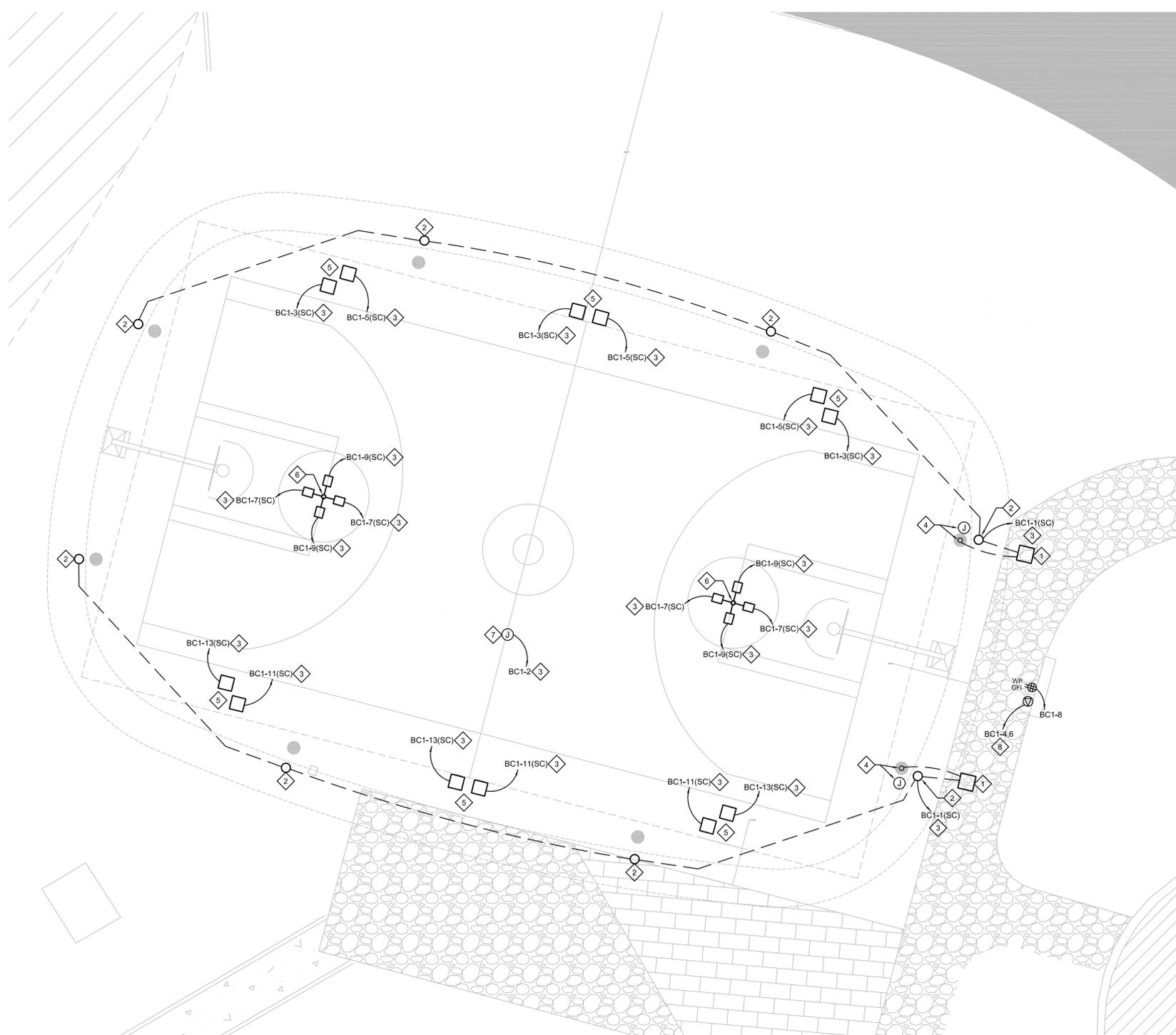
Draftsman	G.Crawf	Phase	DESIGN DEVELOPMENT
GVI Approval	A. Lovelock	Project No.	230164 cz
Client Approval	A. Lovelock	File	
Date	May, 1, 2023	Code	

SCALE

SHEET NUMBER

E001

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1 ELECTRICAL FLOOR PLAN
 SCALE: 1/8" = 1'-0"
 E100

GENERAL NOTES POWER SHEETS: (APPLIES TO ALL POWER SHEETS)

- SEE ALL OTHER PLANS FOR ADDITIONAL DEVICES. SOME POWER CIRCUITING MAY BE ON OTHER PLANS.
- MINIMUM CIRCUIT SIZE IS 2 #12 AND 1 #12 GROUND IN 3/4" CONDUIT. ALL CONDUCTORS SHALL BE 75 DEGREE COPPER THHN INDOOR, THWN FOR EXTERIOR USAGE, COLOR CODED AS PER NEC AND LOCAL AMENDMENTS WITH SIZE, TEMPERATURE, AND VOLTAGE PERMANENTLY PRINTED ON THE JACKET. ALL JOINTS SHALL BE MADE UP USING SELF LOCKING, TWIST-ON, COLOR CODED, SQUARE WIRE SPRING GRAB, LONG SKIRT, WIRE CONNECTORS WITH SWEEP WINGS.
- ALL RECEPTACLES SHALL BE SPEC GRADE, MINIMUM 20 AMP RATED. GFI RECEPTACLES SHALL HAVE TEST BUTTONS WITH INDICATOR LIGHTS. EXTERIOR RECEPTACLES SHALL BE LABELED WEATHER RESISTANT WITH WP COVERS CONFORMING TO WET LOCATION CORD CONNECTION, NEC 406. OVERSIZED COVER PLATES ARE NOT ALLOWED. COORDINATE COLOR WITH OWNER/ARCHITECT.
- ALL EQUIPMENT SHALL HAVE AN INDIVIDUAL LOCAL DISCONNECTING MEANS, EITHER CORDED PLUG AND RECEPTACLE OR SWITCHED DISCONNECT. VERIFY FROM EQUIPMENT SUBMITTED OR RELOCATED IF DIRECT CONNECT OR RECEPTACLE. IF DIRECT CONNECT, PROVIDE SAFETY SWITCH LOCKABLE IN THE OPEN POSITION AS PER NEC. OTHERWISE PROVIDE RECEPTACLE CORD PLUG AS REQUIRED BY EQUIPMENT SUBMITTAL.
- ON CIRCUITS GREATER THAN 20A, FEEDING MULTIPLE PIECES OF EQUIPMENT, PROVIDE FUSED DISCONNECTS (SIZED FOR EQUIPMENT PROTECTING).
- CONTRACTOR IS RESPONSIBLE FOR UPDATING THE CIRCUITING INFORMATION OF ELECTRICAL PANELS BASED UPON THE ACTUAL INSTALLATION.
- CONNECT NO MORE THAN 5 RECEPTACLES TO ANY CIRCUIT. VERIFY AND TRACE RECEPTACLE COUNT PRIOR TO CONNECTING TO EXISTING CIRCUITS.

ELECTRICAL KEYED NOTES:

- ELECTRIC HANDHOLE. REF: 1/E001
- IN-GRADE RECESSED LED LIGHT. REF: SPECIALIST LIGHTING DRAWINGS FOR LOCATION AND SPECIFICATIONS.
- PROVIDE (2) - #8 AND (1) - #8 GND.
- PROVIDE (2) - 2-1/2" CONDUITS FROM HANDHOLE STUBBED INTO BOTTOM OF STRUCTURAL COLUMN. COORDINATE WITH STRUCTURAL FOR CONDUIT IN FOOTING AND PENETRATION OF BASE PLATE. BRANCH CIRCUITS SHALL TRANSITION TO A WET LOCATION LISTED MC CABLE TO ROUTE UP STRUCTURAL COLUMN. PROVIDE A JUNCTION BOX MOUNTED TO STRUCTURE AT THE TOP OF THE STRUCTURAL COLUMN TO TRANSITION BRANCH CIRCUITS BACK TO HARD PIPE.
- SURFACE MOUNTED LED SPORTS LIGHT AT CEILING LEVEL. REF: SPECIALIST LIGHTING DRAWINGS FOR LOCATION AND SPECIFICATIONS.
- PENDANT MOUNTED LED UPLIGHT. REF: SPECIALIST LIGHTING DRAWINGS FOR LOCATION AND SPECIFICATIONS.
- PROVIDE A JUNCTION BOX MOUNTED TO STRUCTURE FOR THE LIGHTING CONTROL SYSTEM GATEWAY. REF: SPECIALIST LIGHTING DRAWING FOR LOCATION AND SPECIFICATIONS.
- PROVIDE (3) - #8 AND (1) - #8 GND. PROVIDE AN "IN-USE" WEATHERPROOF COVER.

ISSUED SETS

Date	Description

REVISIONS

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion
 Hemisfair
 San Antonio, TX. 78205

PROFESSIONAL SEAL

PRELIMINARY REVIEW

NOT FOR REGULATORY APPROVAL, PERMIT OR CONSTRUCTION

AARON T. LOVELOCK
 P.E. REG. NO. 145763
MAY 1, 2023

SHEET NAME

ELECTRICAL FLOOR PLAN

Draftsman	G.Crawf	Phase	DESIGN DEVELOPMENT
GVI Approval	A. Lovelock	Project No.	230164 cz
Client Approval	A. Lovelock	File	
Date	May, 1, 2023	Code	

SCALE

SHEET NUMBER



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E100

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Date	Description

No.	Date	Description
01	05/01/2023	BIDDING SET

Hemisfair Sports Court Pavilion

Hemisfair
San Antonio, TX. 78205

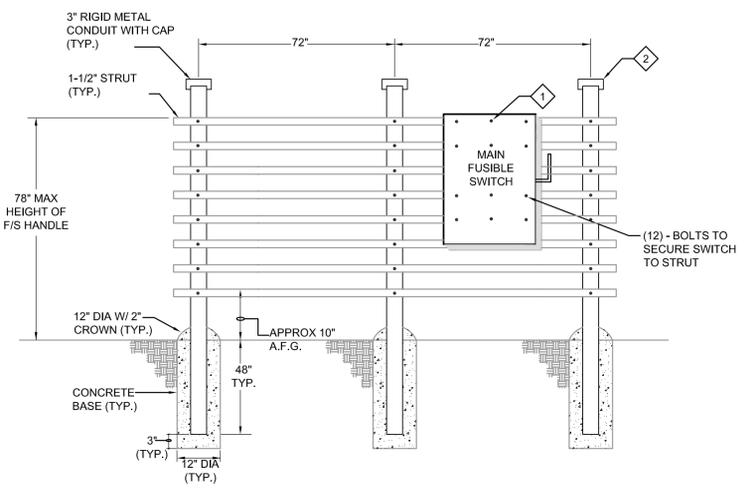
PRELIMINARY REVIEW

NOT FOR REGULATORY APPROVAL, PERMIT OR CONSTRUCTION
 AARON T. LOVELOCK
 P.E. REG. NO. 145763
MAY 1, 2023

ELECTRICAL DETAILS

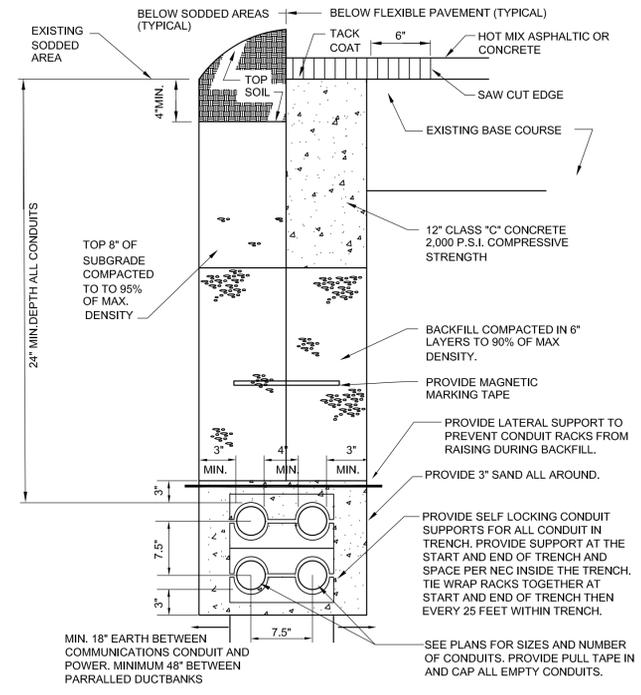
Draftsman	G.Crawf	Phase	DESIGN DEVELOPMENT
GVI Approval	A. Lovelock	Project No.	230164 cz
Client Approval	A. Lovelock	File	
Date	May, 1, 2023	Code	

E601



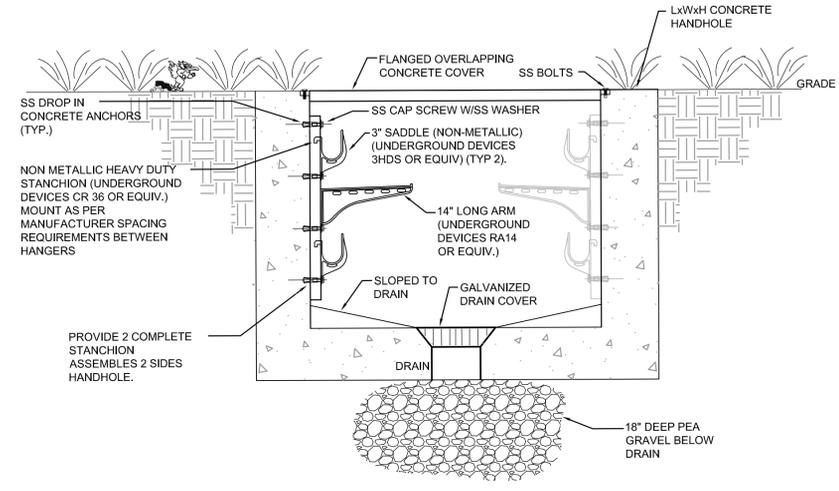
- KEYED NOTES:**
- 1 3/8" DIAMETER GALVANIZED STEEL BOLT DOUBLE-NUTTED ON BOTH ENDS. COAT ALL RAW ENDS WITH GALVANIZED PAINT.
 - 2 FREESTANDING SUPPORT RACK WITH CONCRETE BASE PER POWER COMPANY REQUIREMENTS. (REFER TO CPSE ELECTRIC SERVICE STANDARDS, FIGURE 1800.18.) CONCRETE BASES SHALL BE NOT LESS THAN 8" DIA. AND EXTEND 36" BELOW GRADE AND 6" ABOVE GRADE.

1 EQUIPMENT RACK DETAIL
E601 NOT TO SCALE

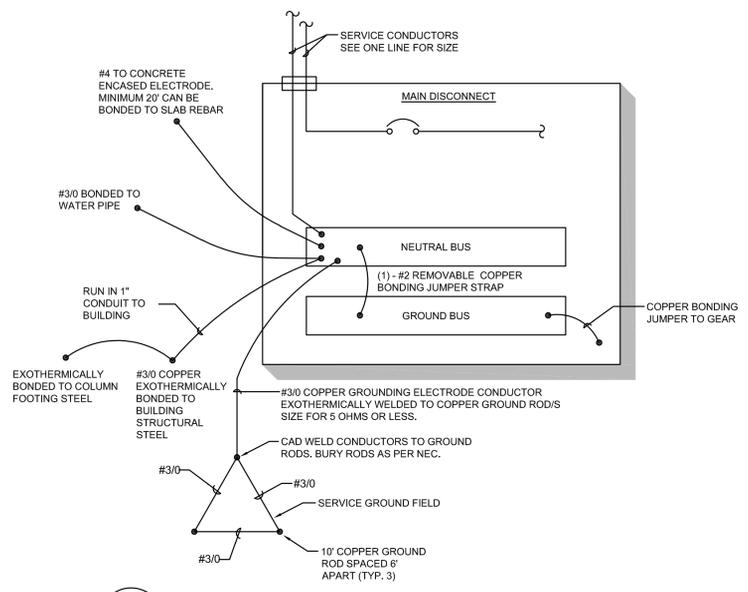


NOTE:
 APPLIES TO ALL BURIED CONDUITS. BACKFILL APPLIES TO ALL AREAS NOT JUST ROADWAYS OR ADJACENT TO ROADWAY.

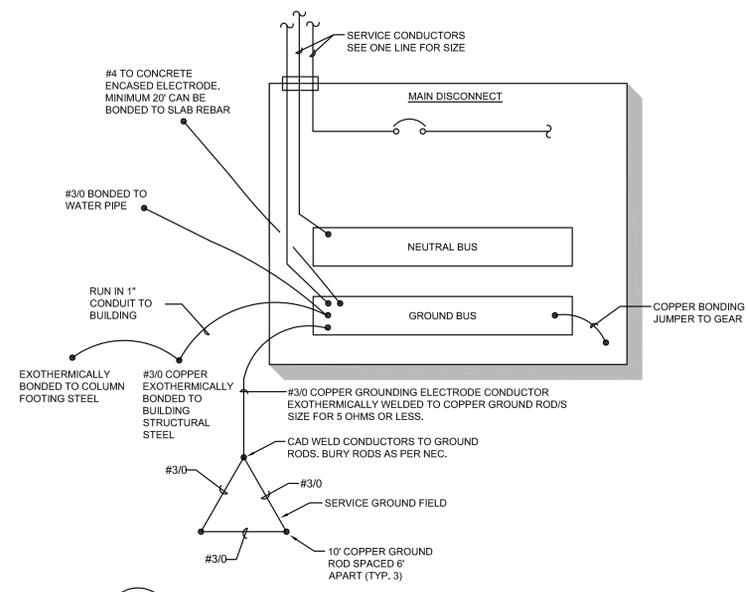
2 CONDUIT BURIAL DETAIL
E601 NOT TO SCALE



3 HANDHOLE DETAIL
E601 NOT TO SCALE



4 GROUNDING AND BONDING DETAIL
E601 SCALE: N.T.S.



5 GROUNDING AND BONDING DETAIL
E601 SCALE: N.T.S.

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